FLOOD INSURANCE STUDY

KANE COUNTY, ILLINOIS
AND INCORPORATED AREAS

Volume 1 of 3

COMMUNITY NAME	COMMUNITY NUMBER
ALGONQUIN, VILLAGE OF	170474
AURORA, CITY OF	170320
BARRINGTON HILLS, VILLAGE OF	170058
BARTLETT, VILLAGE OF	170059
BATAVIA, CITY OF	170321
BIG ROCK, VILLAGE OF	171081
* BURLINGTON, VILLAGE OF	171077
CAMPTON HILLS, VILLAGE OF	171396
CARPENTERSVILLE, VILLAGE OF	170322
EAST DUNDEE, VILLAGE OF	170323
ELBURN, VILLAGE OF	171026
ELGIN, CITY OF	170087
GENEVA, CITY OF	170325
GILBERTS, VILLAGE OF	170326
HAMPSHIRE, VILLAGE OF	170327
* HOFFMAN ESTATES, VILLAGE OF	170107
HUNTLEY, VILLAGE OF	170480
KANE COUNTY (UNINCORPORATED AREAS)	170896
* KANEVILLE, VILLAGE OF	171388
LILY LAKE, VILLAGE OF	171023
MAPLE PARK, VILLAGE OF	171018
MONTGOMERY, VILLAGE OF	170328
NORTH AURORA, VILLAGE OF	170329
PINGREE GROVE, VILLAGE OF	171078
SLEEPY HOLLOW, VILLAGE OF	170331
SOUTH ELGIN, VILLAGE OF	170332
ST. CHARLES, CITY OF	170330
SUGAR GROVE, VILLAGE OF	170333
VIRGIL, VILLAGE OF	171024
WAYNE, VILLAGE OF	170865
WEST DUNDEE, VILLAGE OF	170335

* NO SPECIAL FLOOD HAZARD AREAS IDENTIFIED IN KANE COUNTY

PRELIMINARY

MAY 13, 2011

Federal Emergency Management Agency

FLOOD INSURANCE STUDY NUMBER

17089CV001D

NOTICE TO
FLOOD INSURANCE STUDY USERS

Communities participating in the National Flood Insurance Program have established repositories of flood hazard data for floodplain management and flood insurance purposes. This Flood Insurance Study (FIS) may not contain all data available within the Community Map Repository. It is advisable to contact the Community Map Repository for any additional data.

The Federal Emergency Management Agency (FEMA) may revise and republish part or all of this FIS report at any time. In addition, FEMA may revise part of this FIS by the Letter of Map Revision process, which does not involve republication or redistribution of the FIS. It is, therefore, the responsibility of the user to consult with community officials and to check the Community Map Repository to obtain the most current FIS components.

Initial Countywide FIS Effective Date: December 20, 2002

Revised FIS Report Dates: November 16, 2006
August 3, 2009
To be determined

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FLOOD INSURANCE STUDY
KANE COUNTY, ILLINOIS AND INCORPORATED AREAS

1.0 INTRODUCTION

1.1 Purpose of Study

This Flood Insurance Study (FIS) revises and supersedes the FIS reports and/or Flood Insurance Rate Maps (FIRMs) and/or Flood Hazard Boundary Maps (FHBMs) in the geographic area of Kane County, Illinois, including: the cities of Aurora, Batavia, Elgin, Geneva and St. Charles; the villages of Algonquin, Barrington Hills, Bartlett, Big Rock, Burlington, Campton Hills, Carpentersville, East Dundee, Elburn, Gilberts, Hampshire, Hoffman Estates, Huntley, Kaneville, Lily Lake, Maple Park, Montgomery, North Aurora, Pingree Grove, Sleepy Hollow, South Elgin, Sugar Grove, Virgil, Wayne and West Dundee; and the unincorporated areas of Kane County (hereinafter referred to collectively as Kane County) and aids in the administration of the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973. This study has developed flood risk data for various areas of the county that will be used to establish actuarial flood insurance rates. This information will also be used by Kane County to update existing floodplain regulations as part of the Regular Phase of the National Flood Insurance Program (NFIP), and by local and regional planners to further promote sound land use and floodplain development. Minimum floodplain management requirements for participation in the NFIP are set forth in the Code of Federal Regulations at 44 C.F.R. § 60.3.

The FIS and FIRMs show the flood-hazard information only for the portions of the cities of Aurora, Batavia, Elgin and St. Charles, and the villages of Algonquin, Barrington Hills, Bartlett, East Dundee, Hoffman Estates, Huntley, Maple Park, Montgomery, and Wayne that lie within Kane County. The remaining portions of these communities lie within other counties as indicated in Table 1, "Multi-County Communities." Please see separately published FIS report and FIRM for the portions of the communities that do not lie within Kane County.

Table 1 - Multi-County Communities

Community	Adjacent Counties
Algonquin, Village of	McHenry
Aurora, City of	DuPage
Barrington Hills, Village of	Cook, Lake, McHenry
Bartlett, Village of	DuPage, Cook
Batavia, City of	DuPage
East Dundee, Village of	Cook
Elgin, City of	Cook
Hoffman Estates, Village of	Cook
Huntley, Village of	McHenry

Table 1 - Multi-County Communities (continued)

Community	Adjacent Counties
Maple Park, Village of`	DeKalb
Montgomery, Village of	Kendall
St. Charles, City of	DuPage
Wayne, Village of	DuPage

Aurora, Bartlett, Batavia, St. Charles, and Wayne were included in their entirety in community based FISs. Only information for the portion of the community that lies within Kane County will be shown on the FIRM. The portion of these communities that lie within DuPage County will eventually be added to that county's FIS. However, until this occurs, their community based FISs should be consulted for the portion of those communities in DuPage County.

Note that the villages of Burlington and Kaneville, and the portion of the Village of Hoffman Estates within Kane County have no Special Flood Hazard Areas (SFHAs) identified.

In some states or communities, floodplain management criteria or regulations may exist that are more restrictive or comprehensive than the minimum Federal requirements. In such cases, the more restrictive criteria take precedence and the State (or other jurisdictional agency) will be able to explain them.

1.2 Authority and Acknowledgments

The sources of authority for this FIS are the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973.

The FIS includes the unincorporated areas of, and incorporated communities within, Kane County. Information on the authority and acknowledgments for each jurisdiction included in this FIS, as compiled from their previously printed FIS reports, is shown below.

Pre-Countywide FISs

Village of Algonquin: The hydrologic and hydraulic analyses for the FIS report dated September 16, 1980 (Reference 1) were performed by the U.S. Army Corps of Engineers (USACE), Chicago District, for the Federal Insurance Administration (FIA), under Inter-Agency Agreement No. IAA-H-18-78, Project Order No. 13. That work was completed in October 1979.

City of Aurora:

The hydrologic and hydraulic analyses for the FIS report dated December 1978 (Reference 2) were performed by Harza Engineering Company for the FIA under Contract No. H-3809. This work was completed in October 1976.

The hydraulic analyses for the FIS dated May 15, 1986 (Reference 3) were obtained from the Illinois Department of Transportation (IDOT).

The hydrologic and hydraulic analyses for the FIS dated January 5, 1989 (Reference 4) were taken from the FISs for the City of Aurora and the unincorporated areas of Kane County and DuPage County and from a Soil Conservation Service (SCS) floodplain management study for Indian Creek and tributaries (Reference 4, 5, 6, 7).

The hydrologic and hydraulic analyses for the revised FIS dated March 3, 1997 (Reference 8) were prepared by SCS and IDOT, Division of Water Resources. This work was completed in June 1989.

Village of Bartlett:

The hydrologic and hydraulic analyses for the FIS dated December 15, 1980 (Reference 9) were prepared for the FIA, under Inter-Agency Agreement No. IAA-H-7-76, Project Order No. 19. This work was completed in June 1977.

City of Batavia:

The hydrologic and hydraulic analyses for the FIS report dated March 2, 1981 (Reference 10) were performed by the USACE, Chicago District, for the FIA under Inter-Agency Agreement No. IAA-H-8-78, Project Order No. 13. That work was completed in February 1980.

Village of Carpentersville:

The hydrologic and hydraulic analyses for the FIS report dated February 17, 1981 (Reference 11) were performed by the USACE, Chicago District, for the FIA under Inter-Agency Agreement No. IAA-H-18-78,

Project Order No. 13. That work was completed in December 1979.

Village of East Dundee: The hydrologic and hydraulic analyses for the FIS report dated September 16, 1980 (Reference 12) were performed by the USACE, Chicago District, for the Federal Insurance Administration, under Inter-Agency Agreement No. IAA-H-18-78, Project Order No. 13. This study was completed in October 1979.

City of Elgin: The hydrologic and hydraulic analyses for the FIS report dated 1981 (Reference 13) were performed by the USACE, Chicago District, for the FIA, under Inter-Agency Agreement No. IAA-H-18-78, Project Order No. 13. This study was completed in January 1980.

City of Geneva: The hydrologic and hydraulic analyses for the FIS report dated February 3, 1981 (Reference 14) were performed by the USACE, Chicago District, for the FIA, under Inter-Agency Agreement No. IAA-H-18-78, Project Order No. 13. That work was completed in December 1979.

Village of Hampshire: The hydrologic and hydraulic analyses for the FIS report dated September 2, 1980 (Reference 15) and FIRM dated March 2, 1981 (Reference 16), were prepared by the USACE, Chicago District, for the FIA, under Inter-Agency Agreement No. IAA-H-18-78, Project Order No. 13. That work was completed in September 1979.

The hydrologic and hydraulic analyses for the FIS report dated November 2, 1995 (Reference 17) for Hampshire Creek and Hampshire Creek South No. 1 were prepared by Engineering Enterprise, Inc. That work was completed in April 1992.

Village of Huntley: The hydrologic and hydraulic analyses for the FIS report dated December 15, 1992 (Reference 18) were prepared by the USACE, Chicago District for FEMA, under Inter-Agency Agreement No. EMW-99-E-2739,

Project Order No. 2. These analyses were then revised by a report prepared by Guillou & Associates, Inc., and Haeger & Associates Inc. (Reference 19).

For the revised FIS report dated May 19, 1997 (Reference 20), portions of the hydrologic and hydraulic analyses for the South Branch Kishwaukee River were prepared by Dewberry & Davis using an updated hydraulic analysis prepared by Envirodyne Engineers, Inc., and modified by SDI Consultants, Ltd., for the unincorporated areas of McHenry County FIS (Reference 21). The modified hydraulic analysis was completed in February 1996. Additionally, the hydrologic and hydraulic analyses for Kishwaukee Creek were prepared by Envirodyne Engineers, Inc., for FEMA, under Contract No. EMW-91-C3357. This work was completed in November 1992.

Kane County
(Unincorporated Areas):

The hydrologic and hydraulic analyses for the FIS report dated March 1, 1982 (Reference 5) were prepared by the USACE, Chicago District, for the FIA, under Inter-Agency Agreement No. IAA-H-18-78, Project Order No. 13. That work was completed in June 1980.

The hydrologic and hydraulic analyses for the FIS report dated June 4, 1996 (Reference 22) for Mill Creek were prepared by the USACE, Buffalo District, for FEMA under Inter-Agency Agreement No. EMW-89-E-2994, LMMP No. 89-9. This work was completed in September 1991. Also, the hydrologic and hydraulic analyses for the Ferson/Otter Creek watershed were performed by Christopher B. Burke Engineering, Ltd., for FEMA under Contract No. EMW-90-C-3904. This work was completed in January 1992. The hydrologic and hydraulic analyses for Hampshire Creek and its four tributaries were performed by Engineering Enterprises, Inc. That work was completed in April 1992. The hydrologic and hydraulic analyses for a

portion of Hampshire Creek South were taken from the FIS for the village of Hampshire.

Village of Maple Park: The hydrologic and hydraulic analyses for the FIS report dated August 4, 1987 (Reference 23) were obtained from the U.S. Geological Survey (USGS) publication, *Floods in Maple Park Quadrangle, Northeastern Illinois* (Reference 24).

Village of Montgomery: The hydrologic and hydraulic analyses for the FIS report dated February 1979 (Reference 25) were performed by the Illinois State Water Survey for the FIA, under Contract No. H-3825. That work, which was completed in December 1976, covered all significant flooding sources in the village of Montgomery.

Village of North Aurora: The hydrologic and hydraulic analyses for the FIS report dated September 16, 1980 (Reference 26) were performed by the USACE, Chicago District, for the FIA, under Inter-Agency Agreement No. IAA-H-18-78, Project Order No. 13. That work was completed in November 1979.

Village of Sleepy Hollow: The hydrologic and hydraulic analyses for the FIS report dated December 15, 1982 (Reference 27) were performed by the USACE, Chicago District, for FEMA, under Inter-Agency Agreement No. IAA-H-18-78, Project Order No. 13. That work was completed in August 1979.

Village of South Elgin: The hydrologic and hydraulic analyses for the FIS report dated January 16, 1981 (Reference 28) were performed by the USACE, Chicago District, for the FIA, under Inter-Agency Agreement No. IAA-H-18-78, Project Order No. 13. That work was completed in November 1979.

City of St. Charles: The hydrologic and hydraulic analyses for the FIS report dated March 2, 1981 (Reference 29) were performed by the USACE, Chicago District, for the FIA, under Inter-Agency

	Agreement No. IAA-H-18-78, Project Order No. 13. That work was completed in March 1980.
Village of Sugar Grove:	The hydrologic and hydraulic analyses for the FIS report dated March 4, 1988 (Reference 30) were performed by the USACE, Chicago District, for FEMA, under Inter-Agency Agreement No. EMW-E-1153, Project Order No. 1. That work was completed in August 1985.
Village of Wayne:	The hydrologic and hydraulic analyses for the FIS report dated June 1, 1981 (Reference 31) were performed by the USACE, Chicago District, for the FIA, under Inter-Agency Agreement No. IAA-H-18-78, Project Order No. 13. That work was completed in February 1980.
Village of West Dundee:	The hydrologic and hydraulic analyses for the FIS report dated June 1, 1981 (Reference 32) were performed by the USACE, Chicago District, for the FIA, under Inter-Agency Agreement No. IAA-H-18-78, Project Order No. 13. That work was completed in October 1979.

The authority and acknowledgements for the villages of Barrington Hills, Big Rock, Burlington, Campton Hills, Elburn, Gilberts, Hoffman Estates, Kaneville, Lily Lake, Pingree Grove, and Virgil are not included because there were no previously printed FISs for those communities.

December 20, 2002
Initial Countywide FIS

For the countywide FIS dated December 20, 2002 (Reference 33), revised hydrologic and hydraulic analyses for a portion of Sandy Creek, Pingree Creek and Tyler Creek upstream of Randall Road were prepared for FEMA by Consoer Townsend Envirodyne Engineers, Inc. The hydrologic and hydraulic analyses for the portion of Tyler Creek downstream of Randall Road were provided by Hey and Associates, Inc. The hydrologic analyses for the two studies for Tyler Creek were combined by the IDNR.

November 16, 2006
Revised Countywide FIS

The revised countywide FIS dated November 16, 2006 (Reference 34) included no new hydrologic and hydraulic analyses. The FIS and FIRM for Kane County, Illinois and incorporated areas were revised to include only information for certain communities within the boundaries of Kane County. Portions of the villages of Algonquin, Barrington Hills, and Huntley are located in other counties. Only the Kane County portions of these communities were included in the revised FIS.

August 3, 2009
Revised Countywide FIS

For the August 3, 2009 revision (Reference 35), the hydrologic and hydraulic analyses for the restudy of Indian Creek watershed (consisting of Indian Creek, Indian Creek Prairie Path Run, Selmarten Creek, South Tributary and Tollway Tributary) were completed by V_3 Companies of Illinois, Ltd. (Reference 36).

The hydrologic and hydraulic analyses for the Blackberry Creek watershed (consisting of Aurora Chain of Lakes, Aurora Chain of Lakes Cherry Hills Diversion, Blackberry Creek, East Run, East Run North Branch, East Run North Loop, Elburn Run, Jericho Lake Diversion, Lake Run, Lake Run Main Street Branch, Lake Run Nelson Lake Branch, Lake Run North of I-88 Overflow, Lake Run North of I-88 Overflow East Branch, Lake Run South of I-88 Diversion, Prestbury Branch, Route 38 Branch, Seavey Road Run, Seavey Road Run Green Road Branch, Seavey Road Run Main Street Branch) were completed by the USGS Contract No. EMC-2001-GR-0048.

Planimetric base map information consists of black and white digital orthophotos provided by the Kane County Information Technologies Department, GIS Technologies Division (Reference 37). The digital orthophotos have a 6-inch resolution and were photogrammetrically compiled from aerial photography and obtained during the spring of 2001.

The coordinate system used for the production of the digital FIRMs is Universal Transverse Mercator (UTM), North American Datum of 1983 (NAD 83), Geodetic Reference System 1980 (GRS80) spheroid.

This countywide FIS was performed under the Cooperating Technical Partners (CTP) Partnership Agreement Nos. EMC-2004-GR-0214, EMC-2005-GR-7026, and EMC-2006-CA-7015 between the Illinois Department of Natural Resources (hereafter referred to as IDNR) and the Federal Emergency Management Agency (FEMA), per the Mapping Activity Statement (MAS) Nos. IDNR04-03, IDNR05-20, and IDNR06-10.

To be determined
Revised Countywide FIS

The hydrologic and hydraulic analyses for the Big Rock and Welch Creek watersheds were performed by the Illinois State Water Survey (ISWS) for Kane County (Reference 38). This study was published January 2009.

Planimetric base map information for the 10 affected panels was derived from digital orthophotos provided by the Kane County Information Technologies Department, GIS Technologies Division (Reference 39). Black and White digital orthophotos with a 6-inch pixel resolution were photogrammatically compiled from aerial photography obtained during the spring of 2008.

The coordinate system used for the production of the digital FIRMs is Universal Transverse Mercator (UTM), North American Datum of 1983 (NAD 83), Geodetic Reference System 1980 (GRS80) spheroid.

This Physical Map Revision (PMR) was performed under the Cooperating Technical Partners (CTP) Partnership Agreement No. EMC-2009-CA-7007 between the Illinois State Water Survey and the Federal Emergency Management Agency (FEMA), per the Mapping Activity Statement (MAS) No. ISWS09-07.

1.3 Coordination

Coordination and outreach activities were performed to create a climate of understanding and ownership of the mapping process at the state and local levels. These activities were ongoing throughout the entirety of the project. The purpose of an initial consultation coordination officer (CCO) meeting, or project team meeting, is to discuss the scope of the project. An intermediate CCO meeting, or scoping meeting, is meant to continue outreach and create a climate of understanding throughout the process. A final CCO meeting, or open house, is held with public officials and the general public to review the results of the study.

Pre-Countywide FISs

The dates of the initial and final CCO meetings held for pre-countywide studies for Kane County's incorporated communities are shown in Table 2, "CCO Meeting Dates for Pre-Countywide Studies."

Table 2 - CCO Meeting Dates for Pre-Countywide FISs

Community	Initial CCO Date	Final CCO Date
Algonquin, Village of	December 1977	April 21, 1980
Aurora, City of	*	April 24, 1994
Bartlett, Village of	January 1976	September 15, 1980
Batavia, City of	December 1977	October 21, 1980

* Data not available

Table 2 - CCO Meeting Dates for Pre-Countywide FISs (continued)

Community	Initial CCO Date	Final CCO Date
Carpentersville, Village of	December 1977	September 24, 1980
East Dundee, Village of	December 1977	April 28, 1980
Elgin, City of	December 1977	*
Geneva, City of	December 1977	September 9, 1980
Hampshire, Village of	*	October 28, 1994
Huntley, Village of	August 7, 1990	July 20, 1994
Kane County (Unincorporated Areas)	July 1989	*
Maple Park, Village of	*	August 21, 1986
Montgomery, Village of	*	June 28, 1977
North Aurora, Village of	December 1977	April 16, 1980
Sleepy Hollow, Village of	October 1977	May 27, 1981
South Elgin, Village of	December 1977	July 28, 1977
St. Charles, City of	December 1977	October 14, 1980
Sugar Grove, Village of	July 1983	September 11, 1986
Wayne, Village of	December 1977	January 16, 1981
West Dundee, Village of	December 1977	April 9, 1980

* Data not available

December 20, 2002
Initial Countywide FIS

For the initial Kane County countywide FIS dated December 20, 2002, acknowledgment letters were sent on October 14, 1999 and October 19, 1999.

November 16, 2006
Countywide FIS

For the revised Kane County countywide FIS dated November 16, 2006, no coordination meetings were noted in the FIS.

August 3, 2009
Revised Countywide FIS

The initial CCO meeting was held on February 7, 2005 and was attended by representatives of FEMA, Kane County, the cities of Aurora, Geneva, Elgin, and St. Charles, the Village of Carpentersville and the study contractor (IDNR). This meeting was intended to discuss various issues and concerns for the study area. An intermediate CCO meeting was held on March 11, 2005 in Geneva, Illinois and was attended by representatives from Kane County, the cities of Aurora, Batavia,

Geneva, Elgin, and St. Charles, the villages of Barrington Hills, Burlington, Carpentersville, Huntley, Sugar Grove, and West Dundee and the study contractor (IDNR).

A preliminary FIRM and FIS were prepared by merging effective FIS text, tables, and profiles with new study data. A preliminary Summary of Map Actions (PSOMA) was also prepared for all affected communities. The PSOMA lists pertinent information regarding Letters of Map Change (LOMCs) that will be affected by the issuance of the FIRM (i.e., superseded, incorporated, and revalidated). Preliminary copies of the FIRM, FIS, and SOMA were distributed to community officials for public review and comment.

The results of the study were reviewed at the final CCO meeting held on November 28, 2007 in Elgin and was attended by representatives of Kane County, IDNR, FEMA, and the following communities: Algonquin, Aurora, Barrington Hills, Big Rock, Campton Hills, Carpentersville, East Dundee, Elgin, Geneva, Huntley, Kane County, Maple Park, Montgomery, Pingree Grove, Sleepy Hollow, South Elgin, St. Charles, Virgil, Wayne, West Dundee. Representatives of the non-Kane County communities of Bolingbrook, Darien, Naperville, Oswego, Schaumburg, Waterman and Yorkville were also in attendance. All problems raised at that meeting have been addressed in this study.

To be determined
Revised Countywide FIS

The results of the PMR were reviewed at the final CCO meeting held on _____, in _____, Illinois, and attended by representatives of _____. All problems raised at that meeting have been addressed in this study.

2.0 AREA STUDIED

2.1 Scope of Study

This FIS covers the geographic areas of Kane County including the incorporated areas listed in Section 1.1.

Typically, areas studied by detailed methods are selected with priority given to all known flood hazards and areas of projected development or proposed construction. Approximate analyses are used to study those areas having low development potential or minimal flood hazards.

Tables 3a-3c summarize the history of stream name changes that have occurred since the December 20, 2002 initial countywide FIS.

Table 3a - Stream Name Changes (December 20, 2002 FIS)

Community	Old Name	New Name
Kane County (Unincorporated Areas)	Unnamed Tributary	Harmony Creek
	Unnamed Tributary	Main Street Ditch
	Tributary B	Indian Creek Prairie Path Run
	Randall Road Tributary	Sandy Creek
City of Elgin	Randall Road Tributary	Sandy Creek
Village of Sugar Grove	Tributary No. 1	Welch Creek Tributary No. 1
	Tributary No. 2	Welch Creek Tributary No. 2

Table 3b - Stream Name Changes (August 3, 2009 FIS)

Community	Old Name	New Name
Kane County (Unincorporated Areas)	Blackberry Creek Tributary A	East Run
	Blackberry Creek Tributary B (Cross sections A to J)	Lake Run
	Blackberry Creek Tributary B (Cross sections K, L, Nelson Lake)	Lake Run Nelson Lake Branch
	Blackberry Creek Tributary C	Seavey Road Run
	Blackberry Creek Tributary D	Elburn Run
	Blackberry Creek Tributary E	Prestbury Branch
	Blackberry Creek Tributary F	Lake Run
	Blackberry Creek Tributary G	Jericho Lake Diversion
	Blackberry Creek Tributary H (Cross sections A to C)	Aurora Chain of Lakes
	Bowes Creek Tributary	Bowes Creek South
	Coon Creek	Burlington Creek
	Eakin Creek Tributary	Eakin Creek South
	East Branch	Anderson Road Run North Branch
	Hampshire Creek Tributary	Hampshire Creek South
	Johnson’s Mound Run	Johnsons Mound Run
	Kendall Road Run	Kendall Run

Table 3b (continued) - Stream Name Changes (August 3, 2009 FIS)

Community	Old Name	New Name
Kane County (continued) (Unincorporated Areas)	Kishwaukee River Tributary	Eakin Creek West
	Main Street Ditch	Lake Run Main Street Branch
	Mill Creek Tributary	Mooseheart Creek
	N. Plato Ditch	North Plato Ditch
	Otter Creek Tributary	Otter Creek West
	Stoney Creek	Stony Creek
	Unnamed Creek	Great Western Run
	Unnamed Tributary	Corron Road Run
	Unnamed Tributary to Tyler Creek	Tyler Creek Unnamed Tributary
	Waubansee Creek	Waubonsee Creek
	West Branch	Anderson Road Run
	Welch Creek	Sugar Grove Branch
	Welch Creek Tributary No. 1	Sugar Grove Branch East
	Young's Creek	Youngs Creek
City of Aurora	Blackberry Creek Tributary A	East Run
	Blackberry Creek Tributary H (Cross sections A to C)	Aurora Chain of Lakes
	Blackberry Creek Tributary H (Cross sections D to I)	Aurora Chain of Lakes Cherry Hills Diversion
	Indian Creek Tributary B	Indian Creek Prairie Path Run
	Waubansee Creek	Waubonsee Creek
Village of Campton Hills	Otter Creek Tributary	Otter Creek West
	Silver Glen Road Run	Silver Glen Run
	Stoney Creek	Stony Creek
Village of Elburn	Blackberry Creek Tributary D	Elburn Run
City of Elgin	Otter Creek Tributary	Otter Creek West
	Stoney Creek	Stony Creek
Village of Hampshire	Hampshire Creek Tributary	Hampshire Creek South
	Kishwaukee River Tributary	Eakin Creek West
Village of Huntley	Eakin Creek Tributary	Eakin Creek South
	Kishwaukee River Tributary	Eakin Creek West

Table 3b (continued) - Stream Name Changes (August 3, 2009 FIS)

Community	Old Name	New Name
Village of Huntley	Eakin Creek Tributary	Eakin Creek South
	Kishwaukee River Tributary	Eakin Creek West
Village of Montgomery	Blackberry Creek Tributary G	Jericho Lake Diversion
	Waubansee Creek	Waubonsee Creek
Village of North Aurora	Blackberry Creek Tributary A	East Run
Village of Pingree Grove	Unnamed Tributary to Tyler Creek	Tyler Creek Unnamed Tributary
Village of South Elgin	Stoney Creek	Stony Creek
	Unnamed Tributary to Fox River	Fox River Unnamed Tributary
Village of Sugar Grove	Blackberry Creek Tributary E	Prestbury Branch
	Welch Creek	Sugar Grove Branch
	Welch Creek Tributary No. 1	Sugar Grove Branch East
	Welch Creek Tributary No. 2	Sugar Grove Branch North

Table 3c - Stream Name Changes (To be determined FIS)

Community	Old Name	New Name
Village of Big Rock and Kane County (Unincorporated Areas)	Unnamed	Duffin Drain Tributary 2
	Unnamed	Welch Creek Tributary 1
	Unnamed	Welch Creek Tributary 2
Kane County (Unincorporated Areas)	Unnamed	Welch Creek Tributary 3
	Unnamed	Welch Creek Tributary 4
	Unnamed	Welch Creek Tributary 5
	Unnamed Tributary to Welch Creek	Welch Creek Tributary 6
	Unnamed	Youngs Creek Tributary 1
	Unnamed	Youngs Creek Tributary 2
	Unnamed	Youngs Creek Tributary 3

The streams, or portions of streams, listed in Table 4, "Revised or New Detailed Studies," have new or revised hydrologic and hydraulic analyses for this PMR.

Table 4 - Limits of Revised or New Detailed Study

Stream	Limits of Revised or New Detailed Study
Big Rock Creek	From approximately 43,730 feet upstream of the confluence with Fox River, the Kane/Kendall County Boundary; to approximately 75,030 feet upstream of the confluence with Fox River, approximately the confluence of West Branch Big Rock Creek and East Branch Big Rock
Duffin Drain	From the confluence with Sugar Grove Branch; to approximately 12,750 feet upstream of the confluence with Sugar Grove Branch, approximately the downstream side of Wheeler Road
East Branch Big Rock Creek	Downstream Reach: From the confluence with Big Rock Creek; to approximately 10,810 feet upstream of the confluence with Big Rock Creek, approximately the confluence with Malgren Drain **AND** Upstream Reach: From approximately 55,525 feet upstream of the confluence with Big Rock Creek, approximately the downstream side of Owens Road; to approximately 73,350 feet upstream of the confluence with Big Rock Creek, approximately the confluence with East Branch Big Rock Creek Tributary 2
East Branch Big Rock Creek Tributary 2	From the confluence with East Branch Big Rock Creek; to approximately 8,090 feet upstream of the confluence with East Branch Big Rock Creek, approximately 2,450 feet upstream of Keslinger Road
Malgren Drain	From the confluence with East Branch Big Rock Creek; to approximately 12,425 feet upstream of the confluence with East Branch Big Rock Creek, approximately the downstream side of Swan Road
Sugar Grove Branch	From the confluence with Welch Creek; to approximately 3,565 feet upstream of the confluence with Welch Creek, approximately 1,150 feet downstream of Fay's Lane
Welch Creek	From the confluence with Big Rock Creek; to approximately 89,160 feet upstream of the confluence with Big Rock Creek, approximately the downstream side of Keslinger Road
West Branch Big Rock Creek	From the confluence with Big Rock Creek; to approximately 14,390 feet upstream of the confluence with Big Rock Creek, approximately the downstream side of U.S. Highway 30

The portions of all streams studied in detail and included in this report are listed in Table 5, "Limits of Detailed Study." In some instances, the detailed studies extend beyond the county boundaries. See adjacent counties for more information. Limits of detailed study are also indicated on the Flood Profiles (Exhibit 1) and on the FIRM (Exhibit 2).

Table 5 - Limits of Detailed Study

Flooding Source	Limits of Detailed Study
7th Avenue Creek	From the confluence with Fox River to 10,400 feet above the confluence with Fox River (approximately 2,400 feet above 13th Avenue)
7th Avenue Creek Tributary	From the confluence with 7th Avenue Creek to 1,280 feet above the confluence with 7th Avenue Creek (approximately 325 feet above State Avenue)
Anderson Road Run	From confluence with Blackberry Creek to 5,575 feet above confluence with Blackberry Creek
Anderson Road Run North Branch	From confluence with West Branch to about 5,050 feet above confluence with West Branch
Aurora Chain of Lakes	From confluence with Blackberry Creek to 18,525 feet above confluence with Blackberry Creek (Indian Trail Road)
Aurora Chain of Lakes Cherry Hills Diversion	From confluence with Aurora Chain of Lakes to 4,763 feet above confluence with overflow from East Run (approximately 3,510 feet above bridge on Gilman Natural Trail)
Big Rock Creek	From approximately 43,730 feet upstream of the confluence with Fox River, the Kane/Kendall County Boundary; to approximately 75,030 feet upstream of the confluence with Fox River, approximately the confluence of West Branch Big Rock Creek and East Branch Big Rock Creek
Blackberry Creek	From 63,510 feet above confluence with Fox River (Route 30) to 185,436 feet above confluence with Fox River (approximately 1,225 feet above Route 38)
Bowes Creek	From the confluence with Stony Creek to 31,575 feet above the confluence with Stony Creek (approximately 200 feet above Dittman Road)
Bowes Creek South	From confluence with Bowes Creek to 2,650 feet above confluence with Bowes Creek (approximately 180 feet above Dittman Road)

Table 5 - Limits of Detailed Study (continued)

Flooding Source	Limits of Detailed Study
Brewster Creek	From the confluence with Fox River to 4,400 feet above the confluence with Fox River (approximately 580 feet above Private Nursery Road)
Carpenter Creek	From the confluence with Fox River to 2,000 feet above the confluence with Fox River (approximately 25 feet above Spring Street)
Duffin Drain	From the confluence with Sugar Grove Branch; to approximately 12,750 feet upstream of the confluence with Sugar Grove Branch, approximately the downstream side of Wheeler Road
Eakin Creek	From confluence with South Branch Kishwaukee River to 3,425 feet above confluence with South Branch Kishwaukee River (approximately 2,055 feet above county boundary)
East Run	From the confluence with Blackberry Creek to 20,950 feet above the confluence with Blackberry Creek (approximately 100 feet above Oak Street)
East Branch Big Rock Creek	Downstream Reach: From the confluence with Big Rock Creek; to approximately 10,810 feet upstream of the confluence with Big Rock Creek, approximately the confluence with Malgren Drain **AND** Upstream Reach: From approximately 55,525 feet upstream of the confluence with Big Rock Creek, approximately the downstream side of Owens Road; to approximately 73,350 feet upstream of the confluence with Big Rock Creek, approximately the confluence with East Branch Big Rock Creek Tributary 2
East Branch Big Rock Creek Tributary 2	From the confluence with East Branch Big Rock Creek; to approximately 8,090 feet upstream of the confluence with East Branch Big Rock Creek, approximately 2,450 feet upstream of Keslinger Road
East Run North Branch	From confluence with East Run to 4,622 feet above confluence with East Run (approximately 1,175 feet above inlet structure)
East Run North Loop	From the confluence with East Run to 2,800 feet above the confluence with East Run (approximately 1,100 feet above Orchard Road)
Elburn Run	From the confluence with Blackberry Creek to 16,525 feet above the confluence with Blackberry Creek (approximately 3,950 feet above Keslinger Road)

Table 5 - Limits of Detailed Study (continued)

Flooding Source	Limits of Detailed Study
Ferson Creek	From confluence with Fox River to 74,750 feet above confluence with Fox River (approximately 75 feet above North Avenue)
Fitchie Creek	From confluence with Otter Creek to 26,300 feet above confluence with Otter Creek (approximately 175 feet above Russell Road)
Four Winds Way Creek	From confluence with Fox River to 2,200 feet above confluence with Fox River (approximately 70 feet above State Route 31)
Fox River	From 242,000 feet above Illinois River to 431,300 feet above Illinois River (approximately 21,400 feet above Main Street)
Fox River East Channel	From 253,000 feet above confluence with Fox River to 258,900 feet above confluence with Fox River (approximately 400 feet above New York Street)
Fox River Tributary	From 875 feet above confluence with Fox River Tributary East Branch to 2,440 feet above confluence with Fox River Tributary East Branch (approximately 100 feet above Aucutt Road)
Fox River Tributary East Branch	From confluence with Fox River Tributary to 2,725 feet above confluence with Fox River Tributary (950 feet above Aucutt Road)
Geneva Creek	From confluence with Fox River to 4,130 feet above confluence with Fox River (approximately 60 feet above South Street)
Hampshire Creek	From 15,800 feet above confluence with Burlington Creek to 35,100 feet above confluence with Burlington Creek (approximately 12,400 feet above Rowell Road)
Hampshire Creek South	From confluence with Hampshire Creek to 1.62 miles above confluence with Hampshire Creek (approximately 1,680 feet above Getzelman Road) and from 2.82 miles above the confluence with Hampshire Creek to 3.25 miles about the confluence with Hampshire Creek (approximately .075 miles above Romke Road)
Hampshire Creek Tributary No. 1	From confluence with Hampshire Creek to 1,400 feet above confluence with Hampshire Creek (approximately 740 feet above Industrial Drive)
Hampshire Creek Tributary No. 2	From confluence with Hampshire Creek to 5,900 feet above confluence with Hampshire Creek (approximately 70 feet above Prairie Farm Drive)

Table 5 - Limits of Detailed Study (continued)

Flooding Source	Limits of Detailed Study
Hampshire Creek Tributary No. 3	From confluence with Tributary No. 2 to 1,285 feet above confluence with Tributary No. 2.
Hampshire Creek Tributary No. 4	From 175 feet above the confluence with Hampshire Creek to 745 feet above the confluence with Hampshire Creek
Indian Creek	From confluence with Fox River to 33,350 feet above confluence with Fox River (Fermi Lab Berm)
Indian Creek Prairie Path Run	From the confluence with Indian Creek to 4,580 feet above confluence with Indian Creek (approximately 860 feet above Loreen Drive)
Jelkes Creek	From the mouth at Fox River to 20,260 feet above mouth at Fox River (Sleepy Hollow Road)
Jericho Lake Diversion	From 130 feet below Route 30 to 9,230 feet above Route 30 (approximately 1,560 feet above Jericho Road)
Lake Run	From confluence with Blackberry Creek to 43,000 feet above confluence with Blackberry Creek (approximately 100 feet above Hughes Road)
Lake Run Main Street Branch	From confluence with Lake Run to 6,100 feet above confluence with Lake Run (approximately 2,900 feet above Main Street)
Lake Run Nelson Lake Branch	From confluence with Lake Run to 7,850 feet above confluence with Lake Run (approximately 7,550 above Private Farm Road)
Lake Run North of I-88 Overflow	From confluence with Lake Run to 4,500 feet above confluence with Lake Run
Lake Run North of I-88 Overflow East Branch	From confluence with Lake Run North of I-88 Overflow to 1,875 feet above confluence with Lake Run North of I-88 Overflow
Lake Run South of I-88 Diversion	From confluence with Lake Run to 7,400 feet above confluence with Lake Run
Lords Park Tributary	From the mouth at Poplar Creek to 5,000 feet above the mouth at Poplar Creek (approximately 750 feet above Laurel Street)

Table 5 - Limits of Detailed Study (Continued)

Flooding Source	Limits of Detailed Study
Mahoney Creek	From confluence with Fox River to 12,320 feet above confluence with Fox River (approximately 2,320 feet above Wilson Street)
Malgren Drain	From the confluence with East Branch Big Rock Creek; to approximately 12,425 feet upstream of the confluence with East Branch Big Rock Creek, approximately the downstream side of Swan Road
McKee Road Tributary	From mouth at Mill Creek to 15,900 feet above mouth at Mill Creek (approximately 4,670 feet above Fabyan Parkway)
Mill Creek Tributary 2	From mouth to 2,500 feet above mouth (approximately 1,375 feet above Bridle Creek Drive)
Mill Creek	From mouth to 72,190 feet above mouth (approximately 210 feet above State Route 64)
North Arm Brewster Creek	From mouth to 510 feet above mouth
Norton Creek	From mouth to 18,800 feet above mouth (approximately 2,800 above Dunham Road)
Norton Creek Tributary	From the mouth to 500 feet above the mouth (County Boundary)
Otter Creek	From the confluence with Ferson Creek to 29,750 feet above the confluence with Ferson Creek (approximately 0 feet above Randall Road)
Pingree Creek	From confluence with Tyler Creek to 14,506 feet above confluence with Tyler Creek (approximately 276 feet above U.S. Route 20)
Poplar Creek	From confluence with Fox River to 5,200 feet above the confluence with Fox River (approximately 1,700 feet above Saint Charles Street)
Prestbury Branch	From confluence with Blackberry Creek to 8,500 feet above confluence with Blackberry Creek (approximately 5,600 feet above Winthrop Road)
Route 38 Branch	From confluence with Blackberry Creek to 4,500 feet above confluence with Blackberry Creek (approximately 2,550 feet above Route 38)
Sandy Creek	From mouth to 17,925 feet above mouth (approximately 6,925 feet above Farmers Crossing)

Table 5 - Limits of Detailed Study (continued)

Flooding Source	Limits of Detailed Study
Seavey Road Run	From confluence with Blackberry Creek to 24,200 feet above confluence with Blackberry Creek (approximately 900 feet above Main Street)
Seavey Road Run Green Road Branch	From confluence with Seavey Road Run to 3,150 feet above confluence with Seavey Road Run (approximately 150 feet above Green Road)
Seavey Road Run Main Street Branch	From confluence with Seavey Road Run to 6,100 feet above the confluence with Seavey Road Run (approximately 150 feet above Main Street)
Selmarten Creek	From confluence with Indian Creek to 4,900 feet above confluence with Indian Creek (approximately 1,350 feet above I-88)
Sleepy Creek	From confluence with Fox River to 12,625 feet above confluence with Fox River (State Route 72)
South Tributary	From confluence with Indian Creek to 6,150 feet above confluence with Indian Creek (approximately 3,300 feet above McClure Road)
State Street Creek	From confluence with Fox River to 4,500 feet above confluence with Fox River (approximately 220 feet above 12th Street)
State Street Creek Tributary	From confluence with State Street Creek to 1,650 feet above State Street Creek (approximately 220 feet above 15th Street)
Stony Creek	From confluence with Otter Creek to 27,531 feet above confluence with Otter Creek (approximately 5,256 feet above Crawford Road)
Sugar Grove Branch	From the confluence with Welch Creek; to 17,115 feet upstream of the confluence with Welch Creek, approximately 3,350 feet upstream of Wheeler Road
Sugar Grove Branch East	From the confluence with Sugar Grove Branch to 5,300 feet above confluence with Sugar Grove Branch (approximately 3,700 feet above North-South Runway)
Sugar Grove Branch North	From the confluence with Sugar Grove Branch to 2,900 feet above confluence with Sugar Grove Branch (approximately 0 feet above U.S. Route 30/Granart Road)

Table 5 - Limits of Detailed Study (continued)

Table 5	Limits of Detailed Study
Tollway Tributary	From confluence with Indian Creek to 2,100 feet above the confluence with Indian Creek (approximately 700 feet above Molitor Road)
Tyler Creek	From the confluence with Fox River to 71,400 feet above confluence with Fox River (approximately 220 feet above State Route 72)
Tyler Creek Unnamed Tributary	From the confluence with Tyler Creek to 8,550 feet above the confluence with Tyler Creek (approximately 4,550 feet above Reinking Road)
Union Ditch No. 2	From County Line road to 2,625 feet above County Line Road
Waubonsee Creek	From 25,810 feet above the mouth at Fox River to 38,100 feet above the mouth at Fox River (approximately 580 feet above Montgomery Road)
Welch Creek	From the confluence with Big Rock Creek; to approximately 89,160 feet upstream of the confluence with Big Rock Creek, approximately the downstream side of Keslinger Road
West Branch Big Rock Creek	From the confluence with Big Rock Creek; to approximately 14,390 feet upstream of the confluence with Big Rock Creek, approximately the downstream side of U.S. Highway 30

The streams, or portions of streams, listed in Table 6, "Limits of Approximate Study," have new or revised approximate analyses as part of this PMR.

Table 6 - Limits of Revised or New Approximate Study

Flooding Source	Limits of Revised or New Approximate Study
East Branch Big Rock Creek	Downstream Reach: From approximately Malgren Drain; to approximately 21,000 feet upstream of Malgren Drain, 300 feet upstream of County Line Road (Kane/DeKalb County Boundary) **AND** Middle Reach: From County Line Road extended (Kane/DeKalb County Boundary); to approximately 15,415 feet upstream of County Line Road extended, the downstream side of Owens Road **AND** Upstream Reach: From approximately 2,315 feet upstream of Interstate 88; to approximately 11,000 feet upstream of Interstate 88, approximately 310 feet upstream of Keslinger Road

Table 6 - Limits of Revised or New Approximate Study (continued)

Flooding Source	Limits of Revised or New Approximate Study
Duffin Drain	From Wheeler Road; to approximately 19,415 feet upstream of Wheeler Road, 6,000 feet upstream of Lasher Road
Duffin Drain West	From the confluence with Duffin Drain; to approximately 2,920 feet upstream of the confluence with Duffin Drain, the downstream side of Scott Road
Duffin Drain Tributary 2	From the confluence with Duffin Drain; to approximately 1,675 feet upstream of the confluence with Duffin Drain
Malgren Drain	From Swan Road; to approximately 8,965 feet upstream of Swan Road, approximately 4,065 feet upstream of Lasher Road
Swan Drain	From the confluence with Malgren Drain; to approximately 3,145 feet upstream of the confluence with Malgren Drain, the downstream side of Swan Road
Welch Creek	From Keslinger Road; to approximately 2,265 feet upstream of Keslinger Road
Welch Creek Tributary 1	From U.S. Highway 30; to approximately 19,370 feet upstream of U.S. Highway 30, approximately 385 feet downstream of Lasher Road
Welch Creek Tributary 2	From the confluence with Welch Creek; to approximately 9,630 feet upstream of the confluence with Welch Creek, approximately 370 feet downstream of Scott Road
Welch Creek Tributary 4	From the confluence with Welch Creek; to approximately 9,665 feet upstream of the confluence with Welch Creek, approximately 235 feet downstream of Keslinger Road
Welch Creek Tributary 6	From the confluence with Welch Creek; to approximately 7,150 feet upstream of the confluence with Welch Creek, approximately 600 feet downstream of the Railroad

Table 6 - Limits of Revised or New Approximate Study (continued)

Flooding Source	Limits of Revised or New Approximate Study
Youngs Creek	Downstream Reach: From the confluence with East Branch Big Rock Creek; to approximately 1,480 feet upstream of the confluence with East Branch Big Rock Creek, County Line Road (Kane/DeKalb County Boundary) **AND** Middle Reach: From approximately 2,085 feet downstream of Owens Road (Kane/DeKalb County Boundary); to approximately 775 feet downstream of Owens Road (Kane/DeKalb County Boundary) **AND** Upstream Reach: From County Line Road (Kane/DeKalb County Boundary); to approximately 16,030 feet upstream of County Line Road (Kane/DeKalb County Boundary), 1,300 feet upstream of Keslinger Road
Youngs Creek Tributary 1	From the confluence with Youngs Creek; to approximately 2,135 feet upstream of the confluence with Youngs Creek, approximately 375 feet upstream of Harter Road
Youngs Creek Tributary 3	From the confluence with Youngs Creek; to approximately 2,995 feet upstream of the confluence with Youngs Creek, approximately 1,525 feet upstream of Keslinger Road

For this Revision

This Physical Map Revision incorporates new studies of the Big Rock Creek and Welch Creek watersheds within Kane County. Detailed flood study with limited survey was conducted and supersedes previous studies within the limits provided in Table 4 for the following streams: Welch Creek, Duffin Drain, Sugar Grove Branch (downstream of the existing detailed study), Big Rock Creek, West Branch Big Rock Creek, East Branch Big Rock Creek, and Malgren Drain. Approximate studies were completed for the remaining stream reaches in the watershed within Kane County with limits as indicated in Table 6.

This FIS also provides a history of the incorporation of determination letters issued by FEMA that have resulted in map changes (Letter of Map Revision [LOMR]) since the December 20, 2002 initial countywide FIS. This incorporation is summarized in Tables 7a-c, “Letters of Map Change.”

At the time of this PMR, there were no existing LOMRs affecting the 10 revised panels.

Table 7a - Incorporated Letters of Map Change (December 20, 2002)

LOMC Type	Case Number	Date Issued	Community	Flooding Source	Project Identifier
LOMR	97-05-153P	7/9/1997	Aurora	Indian Creek	Savannah Subdivision
LOMR	97-05-230P	4/21/1997	Batavia	Mills Creek Tributary	Robert's Lane Subdivision
LOMR	94-05-159P	5/20/1994	Batavia	Mills Creek Tributary	Correction of streamline & Zone A
LOMR	92-05-135P	12/22/1992	Batavia	McKee Road Tributary	Requestor: Leder
LOMR	*	5/22/1990	Batavia	*	Requestor: Bergeson
LOMR	*	7/28/2000	Geneva	McKee Road Tributary	Eaglebrook Subdivision
LOMR	02-05-447P	6/12/2002	Geneva	Geneva Creek	Herrington's Trail Subdivision
LOMR-F	98-05-4378A	7/31/1998	Hampshire	Hampshire Creek	Illinois Route 72 at Getzelman Road
LOMR	96-05-165P	6/5/1996	Hampshire	Hampshire Creek South	Hampshire Prairie Subdivision
LOMR	99-05-103P	7/27/1999	Hampshire	Hampshire Creek	Lunt Manufacturing/ Hampshire Creek
LOMR	98-05-203P	8/27/1998	Huntley	Eakin Creek	Eakin Creek Relocation
LOMR	99-05-157P	6/29/2000	Huntley	South Branch of Kishwaukee River	Del Webb's Sun City
LOMR	00-05-061P	6/15/2000	Huntley	Eakin Creek & Eakin Creek Tributary 3	Neighborhood 8 of Del Webb's Sun City
LOMR	98-05-245P	2/19/1999	Kane County (Unincorporated Areas)	Mill Creek Main Channel	Fox Mill Subdivision
LOMR	98-05-203P	8/27/1998	Kane County (Unincorporated Areas)	Eakin Creek	Eakin Creek Relocation

* Data not available

Table 7a (continued) - Incorporated Letters of Map Change (December 20, 2002)

LOMC Type	Case Number	Date Issued	Community	Flooding Source	Project Identifier
LOMR	97-05-3596P	4/29/1998	Kane County (Unincorporated Areas)	Unnamed Tributary to Ferson Creek	Deer Run Creek Subdivision
LOMR	97-05-067P	7/17/1997	Kane County (Unincorporated Areas)	Mill Creek Tributary No. 2	Fox Mill Subdivision, Unit 4
LOMR-F	96-05-311A	9/25/1996	Kane County (Unincorporated Areas)	Mill Creek Tributary No. 2	Foxmill lots Unit 3, 93-95, 102 & 104-107
LOMA	95-05-2366A	11/8/1995	Kane County (Unincorporated Areas)	Fox River	Lots 1, 2, 3, 4, 5 & 6, Block 15 - Algonquin Shores
LOMR-F	96-05-1862A	5/22/1996	Kane County (Unincorporated Areas)	Blackberry Creek Tributary	Lots 19 & 40-43 Victoria Park Subdivision
LOMR	00-05-027P	8/12/2000	Kane County (Unincorporated Areas)	Woods Creek	Boyer Road
LOMR	*	1/6/1986	Kane County (Unincorporated Areas)	Sleepy Creek	*
LOMR	01-05-2373P	12/18/2001	Kane County (Unincorporated Areas)	Fitchie Creek	Russinwood Subdivision
LOMR	01-05-2948P	12/6/2001	Kane County (Unincorporated Areas)	Unnamed Tributary to Ferson Creek	Gilmore Property/ Pinehave Subdivision
LOMR	96-05-113P	5/15/1996	Montgomery	Fox River Tributary, East Branch	Schaffers Green house
LOMR	95-05-279P	1/11/1996	Montgomery	Unnamed Ponding area near Fox River Tributary / Unnamed Wetland	Montgomery Business Park

* Data not available

Table 7a (continued) - Incorporated Letters of Map Change (December 20, 2002)

LOMC Type	Case Number	Date Issued	Community	Flooding Source	Project Identifier
LOMR-F	96-05-1862A	5/22/1996	North Aurora	Blackberry Creek Tributary	Lots 19 & 40-43 Victoria Park Subdivision
LOMR	00-05-047P	7/10/2001	South Elgin	Otter Creek	Thornwood Development
LOMR-F	96-05-594A	6/13/1996	St. Charles	Fox River	Lots 22 & 21 Fox River Townhomes of the Willowgate
LOMR-F	94-05-086A	5/19/1994	St. Charles	Fox River	Lots 3-6 of Fox River Townhomes (Units 252,1258,1260 & 1266 Willowgate Lane)
LOMR-F	93-05-074A	6/14/1993	St. Charles	Fox River	Units 7,8 and 15-18, Fox River Townhomes

Table 7b - Incorporated Letters of Map Change (November 16, 2006 Revision)

LOMC Type	Case Number	Date Issued	Community	Flooding Source	Project Identifier
LOMR	06-05-B010P	5/18/2006	Kane County (Unincorporated Areas) Carpentersville	South Branch Kishwaukee River	Winchester Heights
LOMR	04-05-4080P	8/6/2005	Kane County (Unincorporated Areas) Huntley	Kishwaukee River Tributary	Primepointe Business Park

Table 7c - Incorporated Letters of Map Change (August 3, 2009 Revision)

LOMC Type	Case Number	Date Issued	Community	Flooding Source	Project Identifier
LOMA	96-05-2038A	*	*	*	*
LOMR-F	02-05-0693A	*	*	*	*
LOMR-F	01-05-1170A	3/23/2001	Aurora	Blackberry Creek Tributary H	The Lindens
LOMR-F	01-05-2918A	9/28/2001	Aurora	Blackberry Creek Tributary H	The Lindens, Parcels 1 and 2
LOMR-F	03-05-1227A	1/24/2003	Aurora	Blackberry Creek	The Lindens
LOMR	07-05-5849P	6/27/2008	Elgin	Sandy Creek	Tuscan Woods Subdivision
LOMR	03-05-1473P	7/30/2003	Geneva	McKee Road Tributary	FEMA Initiated - Reissuance
LOMR	06-05-BC30P	12/28/2006	Hampshire	Hampshire Creek Tributary No. 4	Pasquinelli Development
LOMR	08-05-3393P	8/26/2008	Hampshire	Eakin Creek West	Hampshire High School
LOMR	98-05-203P	8/27/1998	Huntley	Eakin Creek and Tributary 3	Eakin Creek Relocation
LOMR	00-05-061P	6/15/2000	Huntley	Eakin Creek and Tributary 3	Del Webb's Sun City, Neighborhood 8
LOMR	99-05-157P	6/29/2000	Huntley	South Branch Kishwaukee River	Del Webb's Sun City
LOMR	02-05-3575P	10/16/2002	Kane County (Unincorporated Areas)	Unnamed Tributary to Ferson Creek	Oak Shadows Subdivision
LOMR	02-05-3913P	12/31/2002	Kane County (Unincorporated Areas)	West Branch East Branch	Anderson Road Project
LOMR-F	03-05-3972A	6/20/2003	Kane County (Unincorporated Areas)	Mill Creek Tributary No. 2	Fox Mill, Unit No. 3
LOMA	96-05-2038A	6/12/96	Kane County (Unincorporated Areas)	Mill Creek Tributary No. 2	Fox Mill Subdivision
LOMR-F	02-05-0693A	1/09/02	Kane County (Unincorporated Areas)	Mill Creek Tributary No. 2	Lots 93-95; 102; 104-107 – Unit 3 Fox Mill

*Data not available

Table 7c (continued) - Incorporated Letters of Map Change (August 3, 2009 Revision)

LOMC Type	Case Number	Date Issued	Community	Flooding Source	Project Identifier
LOMR	03-05-3385P	6/14/2004	Kane County (Unincorporated Areas)	Mill Creek	Woodside Creek Subdivision
LOMR	05-05-0232P	3/15/2005	Kane County (Unincorporated Areas)	Welch Creek	AE Fraesz Property
LOMR	05-05-0235P	3/15/2005	Kane County (Unincorporated Areas)	Unnamed Tributary to Ferson Creek	Patrick Hunter
LOMR	07-05-0508P	11/28/2006	Kane County (Unincorporated Areas)	Unnamed Depressional Area	Fox Creek Subdivision
LOMR	06-05-BP93P	5/16/2007	Kane County Batavia	McKee Road Tributary	McKee Road Tributary Flood Control Project
LOMR	03-05-3985P	12/15/2003	Kane County Elgin	Lord's Park Tributary Poplar Creek	FEMA Initiated - Reissuance
LOMR	03-05-1837P	9/29/2003	Kane County Gilberts	Tyler Creek	Timber Trails
LOMR	06-05BT15P	1/18/2007	Kane County Hampshire	Hampshire Creek South	Hampshire Creek South LOMR
LOMR	03-05-3994P	6/24/2004	Kane County Maple Park	Union Ditch No. 2	The Settlement Subdivision
LOMR	02-05-2627P	12/31/2002	Kane County St. Charles	Norton Creek	Woods of Fox Glen Subdivision
LOMR	05-05-0119P	10/26/2005	Pingree Grove	Tyler Creek Unnamed Tributary	Cambridge Grove
LOMR	02-05-3595P	8/1/2002	Sleepy Hollow West Dundee	Sleepy Creek	Holze Property
LOMR	03-05-1474P	4/17/2003	South Elgin	Otter Creek	Reissuance
LOMR-F	05-05-5511A	2/2/2006	South Elgin	Fox River	River Place
LOMR	07-05-0398P	8/30/2007	St. Charles	Fox River	First Street Redevelopment
LOMR	07-05-0178P	5/1/2008	Sugar Grove	Sugar Grove Branch	Aurora Municipal Airport Master Drainage Study

2.2 Community Description

Kane County is located in northeastern Illinois approximately 50 miles directly west of downtown Chicago. The county is bordered by Cook and DuPage Counties to the east, Kendall County to the south, DeKalb County to the west, and McHenry County to the north.

According to the US Census Bureau, Kane County encompasses 520.44 square miles and had a population of 404,119 in 2000, 317,471 in 1990, and 251,005 in 1970. The population of the county was estimated to be 493,735 in 2006 (Reference 40). Geneva, the county seat, had a population of 19,552 in 2000 (Reference 41). Aurora, the largest city, had a population of 142,990 in 2000. Population is denser in the east, along the Fox River, with more rural/agricultural land uses in the west (Reference 42).

The climate of the area is characterized as humid continental, typified by warm summers and moderately cold winters. The seasons are markedly distinct and generally lag three to five weeks behind the solstices. The proximity of Lake Michigan to the study area has a moderating effect on this climate.

At the Aurora climate station, the average annual temperature is about 48 degrees Fahrenheit (°F) with high temperatures averaging about 59°F and low temperatures averaging about 37°F. January is typically the coldest month and July the warmest month, with average temperatures of 20°F and 72°F, respectively. The lowest temperature on record is -26°F and was recorded in Aurora on January 20, 1985. The highest recorded temperature is 111°F, which occurred in Aurora on June 14, 1936 (Reference 43).

Precipitation within the county occurs as rain, sleet, snow, and hail with an average annual precipitation of 38.39 inches (Reference 43). Approximately sixty percent of annual rainfall occurs from April to September; the annual runoff being approximately 24 inches (Reference 44).

The Fox River basin is the one major drainage basin in Kane County. About sixty percent of the county lies in this drainage basin. The Fox River runs from north to south along eastern Kane County. The topography of the Fox River basin is characterized by rolling morainal hills, marking the northern and western portions of the basin. The land, east of these hills, forms a gently rolling plateau from the Fox River to the eastern boundary of the watershed. These differing landforms produce an asymmetrical topography with the western portion of the basin attaining greater elevations than the eastern portion. Elevation ranges from 630 feet above sea level in the City of Montgomery to 1,065 feet above sea level in Plato Township, on Tower Road. Johnson's Mound in the central part of the county, has an elevation of 898 feet (Reference 42).

The basin's topography is controlled by both subsurface geology and glacial erosion and deposition. The asymmetrical character of the basin is due to the gentle slope of the bedrock units from their highest elevations along the western

boundary down to the eastern boundary. With the recession of the last glacial sheet, large blocks of ice melted to form the Fox Chain of Lakes and other smaller lakes. Few topographical changes in the basin have occurred since glacial time due to low gradients within the basin (Reference 45).

From the northern Illinois state line south to southern Kane County, the terrain is relatively flat and abounds in lakes and low-lying wetlands. The river falls about 110 feet in this 70-mile stretch and the abundant lakes and wetlands obscure the channel between low banks and wide floodplains. Within this 70-mile reach but some distance from the river, topography usually changes to gently undulating prairie and hilly areas with maximum elevations of the basin found at the western boundary of this stretch (Reference 45).

The Blackberry Creek watershed is a 71.2 square mile watershed located in both Kane and Kendall Counties. While the watershed is primarily agricultural, it is experiencing rapid growth in both population and proportion of urban land area. By 2020, both are expected to double from the 1990 condition (Reference 46). Flood damages have increased in the urban areas of the watershed.

The Indian Creek watershed is a 14.7 square mile watershed located mostly in the City of Aurora. Over fifty percent of the watershed has undergone urbanization. A large portion of the open space and wooded area is associated with Fermi Lab, and the remaining agricultural use is now less than twenty percent of the watershed's total land area (Reference 36).

The Big Rock and Welch Creek watershed drains to the Fox River. Welch Creek joins Big Rock Creek 10.3 miles above its confluence with the Fox River. Big Rock Creek joins Fox River at 31 miles above the confluence with the Illinois River, south of the Kane–Kendall County boundary. The Big Rock and Welch Creek watershed is located in Kane, DeKalb, and Kendall counties in northeastern Illinois, and covers a drainage area of 108 square miles at the southwestern Kane County boundary (Reference 38). Within the watershed, 78 percent of the acreage is in row crops. The remainder of the area is made up of rural grassland (11 percent), forest (5 percent), surface water (1 percent), and urban areas (5 percent) (Reference 47). The watershed area includes a small portion of the Village of Elburn as well as the villages of Sugar Grove, Big Rock, and Kaneville, and the area is expected to experience development in the coming years (Reference 38).

2.3 Principal Flood Problems

Flood peaks may increase as urbanization continues to replace agricultural and wooded lands within the watershed. Flooding is sometimes caused (or intensified) by ice jams, which form at bridges and narrow reaches of the river. Flooding has usually resulted either from heavy thunderstorms following a period of prolonged rainfall that has saturated the ground or from a severe storm during snowmelt conditions.

Overbank flood damage generally occurs along the Fox River as well as along some of the smaller creeks in the county. Floods have occurred in the study reaches of the Fox River basin and its tributaries during all seasons of the year.

Kane County has experienced severe flooding. In 1996, 16.9 inches of rain fell on the county in less than a 24-hour period. Flood stages on the Fox River in Aurora exceeded the 1-percent-annual-chance flood event. The Blackberry Creek in Aurora exceeded the 0.2-percent-annual-chance flood event. Several smaller creeks in southern Kane County were also severely flooded. In February 1997, only about 3.0 to 3.5 inches of rain fell on the county. However, the rain fell on snow covered and frozen ground causing significant runoff.

Major floods in Kane County can be documented from records of U.S. Geological Survey (USGS) gage number 05550000 on the Fox River at Algonquin. The drainage area at the gage is 1,403 square miles. Records are continuous from 1915 to the present. Historic floods and the corresponding river stages are shown in Table 8, "Historical Flood Data."

Table 8 - Historical Flood Data

Fox River at Algonquin, Illinois - USGS Gage Number 05550000

Datum of gage is 729.48 feet NGVD 1929

Flood Stage 3 Feet

Date	Peak Streamflow (cfs)	River Stage (feet)
05/22/2004	6,720	3.09
04/02/1979	6,610	4.00
10/03/1987	6,170	3.99
04/23/1993	6,150	3.75
04/01/1916	5,850	4.50
07/05/1938	5,630	4.37
03/16/1929	5,450	4.42
06/17/2000	5,080	3.42
05/23/1996	4,570	3.43

Flooding and damages in the Blackberry Creek watershed area have increased over the past two decades, with major flood damage occurring during the storms of July 1983, July 1996, and February 1997. The storm of July 17-18, 1996 caused damage to over 1,000 homes and over $13 million in damages (Reference 48).

In the Big Rock and Welch Creek watershed, the highest flood observed by local residents in 71 years (Reference 49) occurred after 10.48 inches of rain fell on October 10, 1954. Since the 1954 flood, the largest flood on record occurred on July 18, 1996 when 16.91 inches of rain were recorded at the Aurora College station (Reference 38).

In Batavia, the primary cause of flooding in the Mahoney Creek basin is usually a combination of snowmelt and rainfall. The approximately 2.39 square mile Mahoney Creek watershed is located entirely within Batavia's planning boundaries. Because of extensive urban development and steep overland slopes in the watershed, storm water runoff moves quickly toward the Mahoney Creek Tributary and rapidly through the stream channel itself. Consequently, high intensity, short duration storms tend to produce higher flood flows (Reference 50).

In Hampshire, the primary cause of flooding in the Hampshire Creek basin is usually a combination of snowmelt and rainfall events. Known flood events on Hampshire Creek South occurred in 1960, 1968, 1972, and 1979. Information on historical floods in the area was obtained from a gaging station on Hampshire Creek from the Village of Hampshire.

In Maple Park, the principal flooding problems are caused by the overflow of Union Ditch No. 2 in the southern section of the village.

In Montgomery, severe flooding occurs along the Fox River and the Waubonsee Creek near the Parkview Estates area. The principal causes of the Waubonsee Creek flooding include the channel's inadequate hydraulic capacity and a severe flow restriction caused by a railroad crossing. The flood of August 26, 1972, resulted in several hundred thousand dollars worth of property damage to the Parkview Estates subdivision.

In Sleepy Hollow, the primary cause of flooding in the Jelkes Creek and Sleepy Creek basins is usually a combination of snowmelt and rainfall events. Each creek frequently overflows its banks.

In Sugar Grove, overbank flooding from Welch Creek (now Sugar Grove Branch) was recorded on July 2, 1983 when 6.17 inches of rain fell within a 24-hour period. On June 13, 1981, 3.78 inches of rain fell within a 24-hour period. Both of these rainfall events were recorded at the Aurora Water Department.

In Wayne, the primary cause of flooding in the Norton Creek basin is usually a combination of snowmelt and rainfall events. The Norton Creek Tributary overflowed its banks on June 10, 1967.

Flooding from Mastodon Lake is not limited to the areas adjacent to the lake, but includes a large area west of the lake along Ashland Avenue.

2.4 Flood Protection Measures

The majority of flood protection projects encompass the drainage basins of the Fox River. The remaining projects consist of the construction and maintenance of floodwater retarding structures, channel maintenance, dredgings, channel improvements, and debris removal for specific reaches of streams. There are no new major flood control projects proposed for the Fox River.

Discharges of the Fox River at Aurora are regulated by several dams upstream along the river. Discharges during a flood event are lower than those that would have occurred prior to regulation. The Aurora Dam and the North Aurora Dam are unregulated spillways. Neither dam offers flood protection; their sole purpose is to provide storage for recreational uses. North Aurora Dam was designed by IDOT, Division of Water Resources in 1974 and constructed by the State of Illinois in 1975.

The North Batavia Dam on the Fox River (north of Wilson Street) provides water storage for recreational use but offers no flood protection.

The Carpentersville Dam and Elgin Dam (river mile 71.84) are unregulated spillways. The crest elevation of the spillway and sluiceway is 708.4 feet for the Elgin Dam and 721.0 feet for Carpentersville Dam. Neither of these dams offer flood protection; their sole purpose is to provide water storage for recreational uses.

The Geneva Dam (river mile 58.67) is an unregulated spillway. The dam offers no flood protection; its sole purpose is to provide water storage for recreational uses.

Concrete retaining walls have been built along the banks of the Fox River approximately 450 feet upstream of Montgomery Dam, but offer little protection from high water. The Montgomery Dam itself is a low-level navigation dam, which also offers little or no flood protection.

South Elgin Dam (river mile 68.18) and St. Charles Dam (river mile 60.65) have unregulated spillways. The crest elevation for the spillway and sluiceway is 699.7 feet for South Elgin Dam and 684.3 feet for St. Charles Dame. Their sole purpose is to provide water storage for recreational uses.

Algonquin Dam, on Illinois Route 62 (Chicago Street) (river mile 82.6), is an unregulated spillway. Algonquin Dam offers little or no flood protection; its sole purpose is to provide water storage for recreational purposes.

The Kane County Board has enacted an ordinance on erosion and sedimentation regulating the floodplain and wetland areas (Reference 51). This ordinance states that no floodplain and/or wetland area shall be disturbed, reshaped or otherwise affected by channel relocation, channel deepening, filling or grading of any type, the erection of any structures, or the storage of any materials or equipment, except as permitted by a special use ordinance. The minimum floodplain elevation is defined as that elevation determined from the flood crest profile of the highest flood of record.

3.0 ENGINEERING METHODS

For the flooding sources studied by detailed methods in Kane County, standard hydrologic and hydraulic study methods were used to determine the flood hazard data required for this study. Flood events of a magnitude that are expected to be equaled or exceeded once on the average during any 10-, 50-, 100-, or 500-year period (recurrence interval) have been selected as having special significance for floodplain management and for flood insurance rates. These events, commonly termed the 10-, 50-, 100-, and 500-year floods, have a 10-, 2-, 1-, and 0.2-percent chance, respectively, of being equaled or exceeded during any year. Although the recurrence interval represents the long term, average period between floods of a specific magnitude, rare floods could occur at short intervals or even within the same year. The risk of experiencing a rare flood increases when periods greater than 1 year are considered. For example, the risk of having a flood which equals or exceeds the 1-percent-annual-chance flood in any 50-year period is approximately 40 percent (4 in 10), and, for any 90-year period, the risk increases to approximately 60 percent (6 in 10). The analyses reported herein reflect flooding potentials based on conditions existing in Kane County at the time of completion of this study. Maps and flood elevations will be amended periodically to reflect future changes.

3.1 Hydrologic Analyses

Hydrologic analyses were carried out to establish peak discharge-frequency relationships for each flooding source studied by detailed methods affecting the community.

Pre-Countywide FISs

Each incorporated community within, and the unincorporated areas of, Kane County, with the exceptions of the villages of Barrington Hills, Big Rock, Burlington, Campton Hills, Elburn, Gilberts, Hoffman Estates, Kaneville, Lily Lake, Pingree Grove, and Virgil, has a previously printed FIS report. The hydrologic analyses described in those reports that have not been superseded by new study information are summarized below.

Unit hydrograph characteristics for the Fox River were based on other studies conducted by the USACE for deriving regional unit hydrograph parameters for similar river basins in the northeastern portion of Illinois (Reference 52, 53, 54). Discharge hydrographs were calculated for selected flood events by utilizing precipitation data, runoff coefficients, base flows, and synthetic unit hydrographs. Rainfall data were generated by a statistical analysis of rain gage records. Four-hour increments of a 24-hour storm rainfall, corresponding to frequencies of 10-, 50-, and 100-years were obtained for the U.S. Weather Bureau Technical Paper No. 40 (Reference 55). Rainfall values for the 500-year storm were then extrapolated from values for the lower three frequencies. Sets of these rainfall increments were entered into the HEC-1 model in critical order to obtain peak discharges for the 10-, 2-, 1-, and 0.2-percent-annual-chance flood (Reference 56).

A HEC-1 hydrologic computer model was used to compute discharges for the Fox River basin (Reference 56). The HEC-1 model relates basin characteristics and

rainfall data to stream discharges. The basin characteristics include vegetation, topography, amount and nature of development, and soil types. The hydrologic model was calibrated with the 1973 event. The 1973 event was selected because it had a relatively uniform distribution over the Fox River basin. For the calibration runs, the actual streamflow gaging values of the Fox River gages were used.

A comparison between discharge-frequency analysis at the Fox River gages and the hydrologic model results was conducted to verify the outflow hydrograph. A log-Pearson Type III discharge-frequency analysis was performed for the peak annual series at the gaging stations using the computer skew coefficients 0.62 and 0.00 (Reference 57). The results of the HEC-1 model compare favorably with the analysis, which used 0.0 skew. Peak discharge-frequency determinations were based on an analysis of basin characteristics and rainfall data using the HEC-1 hydrologic computer model with the SCS option for Ferson Creek, McKee Road Tributary, Mill Creek, Norton Creek, Norton Creek Tributary, and Sandy Creek (Reference 56). As with the Fox River, unit hydrograph characteristics for the study area were based on other studies conducted by the USACE (Reference 54). Discharge hydrographs were calculated for selected flood events by utilizing precipitation data, runoff coefficients, base flows, and synthetic unit hydrographs.

Discharge-frequency relationships for the Fox River at Illinois Avenue were prepared by the USGS using regional flood-frequency relationships for streams in northern Illinois (Reference 58). The results were then adjusted to reflect the present channel conditions (Reference 59). Two other similar discharge-frequency curves were prepared for the Fox River, one at the East-West Tollway and the other at St. Peters School, both in Aurora. The locations are upstream and downstream of Illinois Avenue, respectively. Flood peaks originating from storm runoff from the drainage areas between the three locations were estimated using the rational method. In most cases, the local peak discharges will not synchronize with flood peaks on the Fox River that pass through the city. Therefore, it was assumed, for estimating peak discharges on the Fox River, that peak discharges that met the Fox River peak discharges from intervening areas have a magnitude equal to approximately one-half their corresponding peaks for a given recurrence interval.

In areas where the Fox River divides, the discharge was reduced along the main stem (always the west channel). The remainder of the total discharge was assigned to Fox River East Channel.

Within the Village of Montgomery, discharges for the 10-, 2-, 1-, and 0.2-percent-annual-chance floods for the Fox River were developed using a log-Pearson Type III distribution (Reference 57) of 29 years of historical data measured by the Illinois Division of Water Resources gage on the Fox River at Illinois Avenue in Aurora, Illinois. Results of this procedure were in agreement with flood frequency curves from the USGS publication *Floods in Aurora North Quadrangle, Illinois* (Reference 59) which were extrapolated by a straight line curve for the 1- and 0.2-percent-annual-chance intervals. For the tributaries to the Fox River frequency discharges for the 10-, 2-, and 1-percent-annual-chance floods were determined using regional regression equations from the report *Magnitude and Frequency of*

Floods in Illinois (Reference 60). The 0.2-percent-annual-chance flood was determined from linear extrapolation performed on probability paper.

The stream gages used in the hydrologic analyses for the Fox River are listed in the following tabulation:

Flooding Source and Location	Gage Number	Drainage Area (sq. miles)
Fox River		
At McHenry Dam	05549500 (USGS)	1,250
At Algonquin	05550000 (USGS)	1,402
At Dayton	05552500 (USGS)	2,570
At Aurora	05551500 (USGS)	1,705
At Batavia	05551250 (USGS)	1,649
At East Dundee	05550100 (USGS)	1,446
At St. Charles	05551250 (USGS)	1,649
At South Elgin	05551000 (USGS	1,556
At South Elgin	IDOT-DWR	1,500
At Geneva	IDOT-DWR	1,580
At Aurora	IDOT-DWR	1,710

There are no streamflow recording gages on Norton Creek. In order to simulate flows for Norton Creek and Norton Creek Tributary within Wayne, a regional frequency analysis has been completed for the gages in the vicinity of the Norton Creek drainage basin. Thirteen USGS gages with one to twenty years of record in the DuPage River drainage basin provided data for the regional frequency analysis. To enable the regional frequency model to more accurately predict the flows for a small basin, the DuPage data have been augmented by records from gages with small drainage areas. Also included in the study are six gages from DuPage County basin having drainage areas of less than 20.0 square miles and six gages from basins within the region having drainage areas less than 2.0 square miles. The technique for a regional frequency analysis outlined in Bulletin No. 17 from the U.S. Water Resources Council (Reference 57) has been used to calculate the discharges for Norton Creek and Norton Creek Tributary.

There are no streamflow recording gages on Mahoney Creek. In order to simulate flows for Mahoney Creek, a regional frequency analysis has been completed for the gages in the vicinity of the Mahoney Creek drainage basin. Thirteen USGS gages with adequate records in the DuPage River drainage basin provided data for the regional frequency analysis. To enable the regional frequency model to more accurately predict the flows for a small basin, the DuPage data have been augmented by records from gages with small drainage areas. Also included in the study are six gages from the DuPage County basin having drainage areas less than 20.0 square miles and six gages from basins within the region having drainage areas less than 2.0 square miles. The technique for a regional frequency analysis outlined in the *Flood Hydrograph Package* was used along with the HEC-1 rainfall-runoff computer model to calculate the discharges for Mahoney Creek (Reference 56).

For Four Winds Way Creek and Carpenter Creek, peak discharge-frequency determinations were based on analysis of basin characteristics and rainfall data

using the HEC-1 hydrologic computer model (Reference 56). The basin characteristics include vegetation, topography, amount and nature of development, and soil types. As with the Fox River, unit hydrograph characteristics for the study area were based on other studies conducted by the USACE (Reference 53, 54), and discharge hydrographs were calculated for selected flood events by utilizing precipitation data, runoff coefficients, base flows and synthetic unit hydrographs.

For Jelkes Creek and Sleepy Creek, peak discharge-frequency determinations were based on analysis of basin characteristics and rainfall data using the HEC-1 hydrologic computer model (Reference 56) with the SCS option (Reference 61). Discharges for Sleepy Creek were only computed for the 10- and 100-year storms because of the effects of a dam break which was determined to occur on Sleepy Creek. If the dam were to break sooner than assumed for this study, a greater peak discharge would result downstream. Interbasin flow was considered but found to be insignificant on an entire basin analysis of Jelkes Creek and Sleepy Creek.

For 7th Avenue Creek and 7th Avenue Creek Tributary, peak discharge-frequency determinations were based on analysis of basin characteristics and rainfall data using the HEC-1 hydrologic computer model (Reference 56). The basin characteristics include vegetation, topography, amount and nature of development, and soil types. As with the Fox River, unit hydrograph characteristics for the study area were based on others studies (Reference 55, 62), and discharge hydrographs were calculated for selected flood events by utilizing precipitation data, runoff coefficients, base flows, and synthetic unit hydrographs.

Within Kane County, the discharges for Waubonsee Creek were determined by the SWS using regional floodflow equations for northeast Illinois with modifications to account for the unusual bedrock outcroppings near the mouth of the basin. Only the 1-percent-annual-chance discharges were determined.

Estimates of the 10-, 2-, 1-, and 0.2-percent-annual-chance discharges for Waubonsee Creek were made using regional equations for Illinois (Reference 60). These equations were developed by a multiple regression analysis and considered the following basin characteristics: drainage area, main channel length and slope, mean basin elevation, percentage of forest cover, mean annual precipitation, rainfall intensity, area of lakes and ponds, and soil rainfall runoff relationships.

Discharges for the 10-, 2-, and 1-percent-annual-chance floods were plotted on log-normal probability paper, and the 0.2-percent-annual-chance flood discharges were estimated by straight line extrapolation. The 0.2-percent-annual-chance flood discharge is less reliable than the others because the average period of record for stream gages used to prepare the regional equations is approximately 30 years. The reliability of the 1-percent-annual-chance flood discharge is between those of the 2- and 0.2-percent-annual-chance.

The 1-percent-annual-chance discharge value that was used in the FIS for the city of Aurora, dated December 1978, for Waubonsee Creek was also adopted for the revised FIS for the city of Aurora dated May 15, 1986. The analysis did not include the 10-, 2-, or 0.2-percent-annual-chance flood events. To allow for zone

calculations, the 10-percent-annual-chance flood elevation was generated by approximate methods.

The discharges for Union Ditch No. 2 were calculated by the Illinois State Water Survey, which reviewed data from past storms. Hydrologic analyses were carried out to establish peak discharge-frequency relationships.

The discharge values for Brewster Creek and North Arm Brewster Creek were based on the Illinois State Regression Equations modified for urbanized areas of northeastern Illinois (Reference 60).

The discharge information for Poplar Creek was obtained from the Flood Plain Information report for Poplar Creek watershed in which discharge values were determined from the modified Illinois State Regression Equations (Reference 60, 63).

The discharges for Ferson Creek and Otter Creek basin (Otter Creek, Otter Creek West, Fitchie Creek, Bowes Creek, Bowes Creek South and Stony Creek) were developed using the TR-20 hydrology program (Reference 64). These discharges were determined for the 10-, 2-, 1-, and 0.2-percent-annual-chance flood events. The SCS dimensionless unit hydrograph along with the Curve Number method for loss rates was adopted. A log-Pearson Type III discharge-frequency analysis was performed for the peak annual series at the Ferson Creek USGS gaging station located at Randall Road (Reference 57). The computed skew was weighted with the State of Illinois generalized skew coefficient according to Bulletin No. 17 (Reference 57). Rainfall depths were obtained from Bulletin No. 70 for the 10-, 2-, and 1-percent-annual-chance events (Reference 65). The rainfall value for the 0.2-percent-annual-chance storm was then extrapolated from values for the lower three frequencies. The TR-20 model was calibrated to the gage's log-Pearson Type III analysis. Based on IDOT-DWR criteria, future land use for the watershed was determined and applied to the TR-20 model.

A HEC-1 hydrologic computer model was used to compute peak discharges for Hampshire Creek South (Reference 56). The hydrologic model was calibrated on Ferson Creek with the 1968 and 1972 flood events. The events were selected because they had a relatively uniform distribution over the river basin. For the final calibration runs, the actual streamflow gaging values of Ferson Creek were used, since there were no gaging stations on Hampshire Creek South. A comparison between discharge-frequency analysis at Ferson Creek and the hydrologic model results was conducted to verify the outflow hydrograph. A log-Pearson Type III discharge-frequency analysis was performed for the peak annual series at the Ferson gaging station, using the computed skew coefficient 0.0 (Reference 57). The results of the HEC-1 model compared most favorably with the analysis which uses 0.0 skew. The 54.4-square-mile Ferson Creek drainage area was divided into four subareas. The subarea discharges were combined with the development of a composite HEC-1 rainfall runoff model for Ferson Creek. This model simulated the hydrologic behavior of the drainage patterns, storage, and flow restrictions in the river basin. Based on the Ferson Creek parameter recession analysis, similar recession parameters were assumed applicable to Hampshire Creek South. With

these recession parameters as inputs to the HEC-1 model, discharge hydrographs were computed at various locations in Hampshire Creek South.

In some areas, ponding occurs creating storage areas that cause a loss of discharge. There is a large loss of discharge due to a large ponding area, which forms upstream of the Soo Line Railroad because of a small culvert under the railroad, thereby restricting the flow of Hampshire Creek South.

The SCS Technical Release No. 20 was used for the hydrologic analysis of Hampshire Creek and Hampshire Creek Tributary No. 1 (Reference 64). Rainfall parameters used in the TR-20 model were taken from Bulletin 70 for the 10- and 1-percent-annual-chance recurrence interval storms of 3-, 6-, 12-, 18-, and 24-hour durations using the appropriate rainfall distributions published in Circular 173, *Time Distributions of Heavy Rainstorms in Illinois* (Reference 66). Watershed subareas were determined by review of existing topographic information, aerial photographs, and field reconnaissance. Only those drainage structures passing beneath railroad embankments were taken into consideration for the effects of storage and attenuation of flows.

The discharge-frequency analysis for Mill Creek was performed using stream gage records with Weibull Plotting Positions method. Although there are no USGS gaging stations located on Mill Creek, a staff gage was in place on Mill Creek at Kaneville Road from 1960 to 1979. The record from this gage was used to develop the discharge-frequency relationship, and in combination with a rating curve developed at the bridge, a discharge-frequency curve was produced.

The discharges for Geneva Creek were determined utilizing the USACE HEC-1 hydrologic model (Reference 56). Using 24-hour rainfall data obtained from the Weather Bureau (Reference 55) one-hour values, in critical order, were entered into the HEC-1 model of the basin to determine the 10-, 2-, 1-, and 0.2-percent-annual-chance peak discharge. After the discharges were determined, a flood routing was performed using the HEC-1 computer program at the Chicago and North Western railroad yard crossing with Geneva Creek. This routing was performed to determine the volume of water that would be retained by the structure due to the inadequate sizing of the culvert within the structure. It was determined that this structure would alter flows for the 2-, 1-, and 0.2-percent-annual-chance events.

For State Street Creek and State Street Creek Tributary, peak discharge-frequency determinations were based on an analysis of basin characteristics using the State of Illinois Regression Equations (Reference 60). The basin characteristics include vegetation, topography, amount and nature of development, and soil types.

There are no USGS gaging stations on Sugar Grove Branch (referenced at time of study as Welch Creek) and no previously developed discharge frequency information was available to the study contractor. Therefore, a hydrologic model of Sugar Grove Creek and Sugar Grove Branch East (referenced at time of study as Welch Creek Tributary No. 1) was developed using the USACE HEC-1 flood hydrograph package (Reference 56). The 36.5-square mile Sugar Grove Branch drainage basin was divided into 22 subareas. Peak discharges for each subarea were

determined by using the SCS method available with the HEC-1 computer program. The model includes storage routings at the Burlington Northern Railroad culvert, at the Aurora Airport's east-west runway culvert on Sugar Grove Branch and at the Aurora Airport's north-south runway culvert on Sugar Grove Branch East. The culverts were modified using the Modified Puls Routing procedures. The stage-area relationships that describe the storage characteristics of the basin upstream of the culverts were developed using the USGS Sugar Grove quadrangle map (Reference 67). The routings resulted in flow reductions of up to 60 percent. (Please note that this information applies only to the area south of the railroad. The detailed study of Sugar Grove Branch upstream of the railroad was superseded as a result of LOMR 07-05-0178P. See "August 3, 2009 Revised Countywide FIS" below.)

The 50-, 10-, 1-, and 0.2-percent-annual-chance discharges for Jericho Lake Diversion were determined using the SCS TR-20 computer program (Reference 64). This model was checked for reasonableness against the historic flood of 1983.

Discharges for Lord's Park Tributary, within the city of Elgin, were determined using regional equations.

The hydrologic analysis of Mastodon Lake used the SCS TR-20 computer program as well. Runoff curve numbers and time of concentration parameters for Mastodon Lake used in the SCS TR-20 model were determined by review of aerial photographs, available soils information, topographic maps and field interpretation.

December 20, 2002
Initial Countywide FIS

The hydrology for Tyler Creek, Pingree Creek, and a portion of Sandy Creek was revised to define the peak flows in the area being studied. The revisions consisted of subdividing the watershed into smaller areas, adding channel and reservoir routings, using the State of Illinois Bulletin 70 precipitation amounts, and incorporating interbasin flow between Tyler Creek and Eakin Creek. The USACE HEC-1 computer program was used in the hydrologic modeling. The SCS Curve Number and Unit Hydrograph methods were used.

A HEC-1 model was used to compute discharges on Eakin Creek (Reference 68).

November 16, 2006
Revised Countywide FIS

No new or revised hydrologic studies were incorporated into the November 16, 2006 revised countywide FIS.

August 3, 2009
Revised Countywide FIS

The hydrologic analysis for the streams located within the Blackberry Creek watershed (Aurora Chain of Lakes, Aurora Chain of Lakes Cherry Hills Diversion, Blackberry Creek, East Run, East Run North Branch, East Run North Loop, Elburn Run, Lake Run, Lake Run Main Street Branch, Lake Run Nelson Lake Branch, Lake Run North of I-88 Overflow, Lake Run North of I-88 Overflow East Branch, Lake Run South of I-88 Diversion, Prestbury Branch, Route 38 Branch, Seavey Road Run, Seavey Road Run Green Road Branch, Seavey Road Run Main Street Branch) was revised by the USGS (Reference 48) to define peak flows. The 71.16 square mile drainage area of the Blackberry Creek watershed was divided into 49 subbasins. Flood-hazard analyses were performed for only the Kane County portion of the watershed.

The Hydrological Simulation Program - FORTRAN (HSPF) (Reference 69) was used to perform the hydrologic modeling for the Blackberry Creek watershed. Streamflow data was available at two locations in the watershed: the USGS streamflow-gaging station at Blackberry Creek near Yorkville (station 0551700), located close to the downstream end of the watershed; and the USGS streamflow gaging station at Blackberry Creek near Montgomery (station 05551675), located at the Jericho Road Bridge crossing. Utilizing the annual maximum series (AMS) determined from simulated streamflow records at various locations in the watershed from the hydrologic model, flood-frequency analysis was used to estimate flood quantiles. Procedures for the flood-frequency analysis followed the recommendations described in Bulletin 17B. The frequency analysis was done with the PEAKFQ program (Reference 70). The 1- and 0.2-percent-annual-chance floods determined in this analysis were then used in the hydraulic model analysis.

The hydrologic analysis for the Indian Creek watershed (Indian Creek, Indian Creek Prairie Path Run, Selmarten Creek, South Tributary, and Tollway Tributary) was completed by V_3 Companies of Illinois, Ltd (Reference 36). Precipitation data for the study was taken from the Huff Bulletin 70 (Reference 65) rainfall depths and Circular 173 distributions (Reference 66).

The Indian Creek watershed was modeled in two separate, but dependent sections. The northern portion of the Indian Creek watershed was modeled with the USACE HEC-1 hydrologic computer program (Reference 71). The computed HEC-1 output hydrographs were input into the northern FEQ hydraulic model. The southern portion was modeled conventionally with the resultant HEC-1 discharges at specific locations being input into the southern HEC-RAS hydraulic model. The hydrologic results for the entire Indian Creek watershed were computed for the 10-, 2-, 1-, and 0.2-percent-annual-chance storm events for durations of 3, 6, 12, 24, 48, and 72 hours (Reference 36).

LOMR 07-05-0178P should be referenced for hydrologic information for Sugar Grove Branch, Sugar Grove Branch East, and Sugar Grove Branch North (referenced in the LOMR as Welch Creek, Welch Creek Tributary No. 1, and

Welch Creek Tributary No. 2). The portion of the pre-countywide Sugar Grove Branch study area upstream of the railroad was superseded by this LOMR.

To be determined
Revised Countywide FIS

For this PMR, the hydrologic analysis for Big Rock and Welch Creek watershed was completed by the Illinois State Water Survey for Kane County in December 2008. The Big Rock Creek/south Kane-Kendall County border constitutes the downstream limit of the hydrologic study.

The Big Rock and Welch Creek watershed hydrology was modeled using HEC-HMS version 3.2 (Reference 72) to determine the 1-percent-annual-chance discharge. The analysis was performed using the SCS Curve number loss method, Clark Unit Hydrograph translation method, and Muskingum Cunge and Modified Puls routing calculations.

A precipitation gage and four stage gages captured data for the September 12-14, 2008 flood. The model was calibrated to this event, and flood discharge calculations were made using the slope-area method (Reference 73).

A summary of the drainage area-peak discharge relationships for all the streams studied by detailed methods is shown in Table 9, “Summary of Discharges.”

Table 9 - Summary of Discharges

Flooding Source and Location	*Drainage Area (square miles)*	*Peak Discharges (cubic feet per second)* 10-Percent-Annual-Chance	2-Percent-Annual-Chance	1-Percent-Annual-Chance	0.2-Percent-Annual-Chance
7th AVENUE CREEK					
At mouth	2.9	158	258	303	429
Approximately 6,864 feet from mouth	1.8	110	185	226	317
7th AVENUE CREEK TRIBUTARY					
At confluence with 7th Avenue Creek	0.6	103	192	246	347
ANDERSON ROAD RUN	*	*	*	*	*
ANDERSON ROAD RUN NORTH BRANCH	*	*	*	*	*
AURORA CHAIN OF LAKES					
At confluence with Blackberry Creek (approximately 190 feet upstream of Jericho Road)	4.1	104	430	621	1,449
At intersection with Prairie Street	3.7	154	543	772	1,716
Aurora Chain-of-Lakes at intersection with Gilman Natural Trail	3.4	118	267	359	667
Aurora Chain of Lakes Orchard Road Overflow at intersection with Orchard Road	2.8	5	28	98	364
Approximately 670 feet upstream of Orchard Road	2.7	134	293	337	453
At intersection with Galena Road	2.3	219	425	533	829
Approximately 1100 feet upstream of Illinois Avenue	1.8	125	258	331	544
AURORA CHAIN OF LAKES CHERRY HILLS DIVERSION					
Aurora Chain of Lakes Cherry Hills Diversion at intersection with Gilman Natural Trail	3.6	4	280	438	1,207

*Data not available

Table 9 - Summary of Discharges (Continued)

Flooding Source and Location	Drainage Area (square miles)	Peak Discharges (cubic feet per second) 10-Percent-Annual-Chance	2-Percent-Annual-Chance	1-Percent-Annual-Chance	0.2-Percent-Annual-Chance
BIG ROCK CREEK					
Just Downstream of Welch Creek Confluence	104.4	*	*	12,403	*
Just Downstream of East Branch/West Branch Confluence	60.7	*	*	7,990	*
BLACKBERRY CREEK					
At intersection with US Highway 30	57.1	1,325	2,302	2,808	4,218
At confluence with Aurora Chain of Lakes (approximately 190 feet upstream of Jericho Road)	52.4	1,347	2,373	2,910	4,421
Approximately 80 feet downstream of Burlington Railroad	51.4	1,497	2,465	2,952	4,286
At upstream of confluence with East Run and approximately 300 feet upstream of Galena Road	45.9	1,401	2,286	2,742	3,984
At confluence with Lake Run (approximately 1800 feet downstream of Illinois Route 56)	31.9	1,037	1,681	2,003	2,875
At confluence with Prestbury Branch (approximately 2740 feet upstream of Illinois Route 56)	27.8	995	1,637	1,961	2,847
Approximately 140 feet upstream of Ke-De-Ka Road	25.5	1,003	1,675	2,018	2,961
Approximately 4140 feet downstream from Illinois Route 47	23.5	992	1,670	2,017	2,976
Approximately 550 feet upstream of Scott Road (90 feet upstream of junction with Seavey Road Run)	15.0	719	1,221	1,477	2,189
Approximately 240 feet upstream of Interstate 88	13.4	717	1,261	1,545	2,348
Approximately 50 feet upstream of Illinois Route 47	11.2	634	1,120	1,376	2,097

*Data not available

Table 9 - Summary of Discharges (Continued)

Flooding Source and Location	Drainage Area (square miles)	Peak Discharges (cubic feet per second) 10-Percent-Annual-Chance	2-Percent-Annual-Chance	1-Percent-Annual-Chance	0.2-Percent-Annual-Chance
BLACKBERRY CREEK (continued)					
At confluence with Elburn Run (approximately 3200 feet upstream of Smith Road)	7.0	316	537	651	966
Approximately 125 feet upstream of Hughes Road	6.0	303	523	637	956
At intersection with a private road. The private road connects to Keslinger Road from south and approximately 250 feet east of Deneali Road intersection	4.8	351	628	772	1,174
Approximately 670 feet downstream of BCNW Railroad	3.1	326	561	677	985
At confluence with Route 38 Branch (approximately 1500 feet downstream of Pouley Road and southeastern to the intersection of Illinois Route 38 and Pouley Road)	1.0	177	310	376	551
BOWES CREEK					
At confluence with Stony Creek	8.2	246	542	617	1,354
Approximately 4,660 feet above confluence with Stony Creek	7.7	239	526	600	1,313
At Corron Road	6.6	203	433	492	995
At Crawford Road	5.4	210	451	515	1,015
Above confluence of Bowes Creek Tributary	2.9	173	357	406	745
At Dittman Road	2.5	109	223	251	456
BOWES CREEK SOUTH					
At confluence with Bowes Creek	1.8	94	194	217	394
At Dittman Road	1.7	87	177	199	392
BREWSTER CREEK					
At confluence with Fox River	17.7	162	265	327	482
CARPENTER CREEK					
At confluence with Fox River	1.5	331	531	669	918

Table 9 - Summary of Discharges (Continued)

Flooding Source and Location	Drainage Area (square miles)	Peak Discharges (cubic feet per second) 10-Percent-Annual-Chance	2-Percent-Annual-Chance	1-Percent-Annual-Chance	0.2-Percent-Annual-Chance
DUFFIN DRAIN					
At US 30	8.1	*	*	979	*
At Lasher Road	2.5	*	*	422	*
EAKIN CREEK					
At confluence with South Branch Kishwaukee River	14.9	540	1,059	1,383	1,995
EAST BRANCH BIG ROCK CREEK					
Just Downstream of Malgren Drain Confluence	31.7	*	*	4,386	*
Just Downstream of Youngs Creek Confluence	22.9	*	*	3,473	*
Just Downstream of East Branch Big Rock Tributary 2 Confluence	3.7	*	*	679	*
At Keslinger Road	0.7	*	*	154	*
EAST BRANCH BIG ROCK CREEK TRIBUTARY 2					
At Keslinger Road	0.4	*	*	104	*
EAST RUN					
At confluence with Blackberry Creek (approximately 520 feet downstream of Hankes Road)	4.5	317	580	684	989
Approximately 580 feet upstream of Indian Trail Road	3.6	360	660	784	1,149
Approximately 370 feet downstream of culverts on Orchard Road	*	219	367	484	600
Approximately 50 feet upstream of inflow point to the pond by auto-dealers North East of I-88 Tollway and Orchard Road	*	322	588	643	921
On East Run approximately 490 feet upstream of inflow point to AutoDealers' Pond	*	257	417	486	641
Approximately 2500 feet downstream of Foxhill Lane	1.8	229	367	425	549
Approximately 120 feet upstream of Oak Street	1.0	214	341	400	546

*Data not available

Table 9 - Summary of Discharges (Continued)

Flooding Source and Location	Peak Discharges (cubic feet per second) Drainage Area (square miles)	10-Percent-Annual-Chance	Flooding Source and Location	Drainage Area (square miles)	10-Percent-Annual-Chance
EAST RUN NORTH BRANCH					
On East Run North Branch approximately 250 feet upstream of inflow point to AutoDealers' Pond	*	65	121	157	279
EAST RUN NORTH LOOP					
On North Loop approximately 150 feet south of culvert outlet by I-88 Tollway	*	213	414	447	734
ELBURN RUN					
At confluence with Blackberry Creek (approximately 3200 feet upstream of Smith Road)	2.6	416	750	918	1,373
At intersection with Hughes Road	1.8	281	494	599	879
Approximately 146 feet upstream of Keslinger Road	0.8	105	149	166	206
FERSON CREEK					
At mouth	54.5	1,959	3,486	4,020	6,430
At Randall Road	51.2	1,296	2,700	3,075	6,020
At Bolcum Road	46.3	1,295	2,662	3,027	5,649
Just upstream of confluence of Otter Creek	11.3	342	740	843	1,641
At Burr Road	11.2	342	740	843	1,641
Approximately 2,300 feet upstream of Burr Road	11.0	421	492	524	621
Approximately 3,350 feet downstream of Denker Road	10.6	202	448	516	1,034
At Denker Road	9.0	205	489	563	1,034
Just upstream of confluence of Ferson Creek Tributary	8.6	211	584	680	1,341
At Burlington Road	6.1	184	450	512	971
Just above Lake Campton	6.1	208	456	517	980
Approximately 2,430 feet above Lake Campton	4.5	178	391	443	840
At Retreat Court	3.4	156	342	387	735
At the Great Western Trail	2.0	118	260	294	558

*Data not available

Table 9 - Summary of Discharges (Continued)

Flooding Source and Location	Drainage Area (square miles)	Peak Discharges (cubic feet per second) 10-Percent-Annual-Chance	2-Percent-Annual-Chance	1-Percent-Annual-Chance	0.2-Percent-Annual-Chance
FITCHIE CREEK					
At confluence with Otter Creek	7.2	178	375	433	834
At Bowes Road	6.8	144	300	337	619
At Koshar Circle	5.6	127	265	298	547
Approximately 2,770 feet upstream from Nestler Road	5.0	120	250	313	515
At Russell Road	3.5	77	191	220	447
FOUR WINDS WAY CREEK					
At mouth of Fox River	1.8	181	297	357	561
FOX RIVER					
Upstream of U.S. 30 and downstream of Ashland Avenue	1,710	12,100	17,050	18,700	24,100
Approximately 1.2 miles downstream of North Avenue	1,705	12,100	17,000	18,600	24,100
At Aurora Dam	1,705	5,950	8,400	9,180	11,900
At North Aurora	1,680	8,565	12,770	14,350	18,760
At confluence of Mill Creek	1,670	8,565	12,770	14,350	18,760
Approximately 319,757 feet from mouth	1,649	7,535	11,225	12,250	16,875
At River Station 294,500	1,629	8,500	12,500	13,500	17,630
At Geneva Dam	1,580	7,535	11,225	12,250	16,875
Approximately 356,400 feet from mouth	1,568	7,535	11,225	12,250	16,875
Approximately 359,964 feet from mouth	1,556	6,870	9,965	11,350	14,680
Just upstream of confluence of Norton Creek	1,540	7,535	11,225	12,250	16,875
At U.S. Route 20	1,532	6,870	9,965	11,305	14,680
At Lawrence Avenue	1,509	5,910	8,950	10,540	13,475
Approximately 8,400 feet upstream of confluence of Jelkes Creek	1,446	5,910	8,950	10,540	13,475
At Carpentersville Dam	1,425	5,775	8,345	10,095	12,525
At Algonquin approximately 428,541 feet from mouth	1,403	5,480	7,990	9,690	11,800
Approximately 7,000 feet downstream of upstream county boundary	1,390	5,775	8,345	10,095	12,525

Table 9 - Summary of Discharges (Continued)

Flooding Source and Location	*Drainage Area (square miles)*	*Peak Discharges (cubic feet per second)* 10-Percent-Annual-Chance	2-Percent-Annual-Chance	1-Percent-Annual-Chance	0.2-Percent-Annual-Chance
FOX RIVER EAST CHANNEL					
At Aurora Dam	1,705	6,150	8,600	9,420	12,200
FOX RIVER TRIBUTARY					
Upstream of confluence with Fox River	1.9	134	282	360	510
FOX RIVER TRIBUTARY (EAST BRANCH)					
Upstream of confluence with Fox River Tributary	0.3	25	56	75	105
GENEVA CREEK					
Just downstream of the Chicago and North Western railroad yard	1.2	323	521	539	689
At South Street	1.1	305	466	568	784
HAMPSHIRE CREEK					
Approximately 80 feet downstream of confluence of Hampshire Creek South	5.8	745	*	1,406	*
Approximately 45 feet downstream of confluence of Hampshire Creek South	3.8	618	*	1,288	*
Approximately 1,961 feet downstream of State Street	3.7	553	*	1,153	*
Approximately 107 feet upstream of State Street	3.5	533	*	1,153	*
Approximately 506 feet upstream of State Street	3.4	446	*	990	*
Approximately 1,650 feet upstream of Rowell Road	2.1	328	*	819	*
Approximately 3,101 feet upstream of Rowell Road	1.5	240	*	557	*
Approximately 4,437 feet upstream of Rowell Road	1.5	188	*	469	*
Approximately 4,000 feet downstream from Soo Line Railroad	1.1	166	*	415	*

*Data not available

Table 9 - Summary of Discharges (Continued)

Flooding Source and Location	Drainage Area (square miles)	Peak Discharges (cubic feet per second) 10-Percent-Annual-Chance	2-Percent-Annual-Chance	1-Percent-Annual-Chance	0.2-Percent-Annual-Chance
HAMPSHIRE CREEK (continued)					
Approximately 1,927 feet downstream of Soo Line Railroad	0.8	115	*	250	*
Approximately 210 feet downstream of Soo Line Railroad	0.5	53	*	139	*
Approximately 18 feet upstream of Soo Line Railroad	0.5	61	*	160	*
HAMPSHIRE CREEK SOUTH					
At confluence with Hampshire Creek	2.0	170	237	276	336
At Soo Line Railroad	2.0	260	420	556	822
At Romke Road	1.1	138	*	297	*
HAMPSHIRE CREEK TRIBUTARY NO. 1					
At confluence with Hampshire Creek	0.6	64	*	117	*
Approximately 717 feet upstream of Industrial Drive	0.5	64	*	117	*
HAMPSHIRE CREEK TRIBUTARY NO. 2					
At confluence with Hampshire Creek	0.6	94	*	294	*
Approximately 409 feet downstream of Allen Road	0.3	48	*	150	*
Approximately 651 feet downstream of Penstemon Lane	0.2	29	*	89	*
Approximately 566 upstream of Penstemon Lane	0.1	19	*	58	*
HAMPSHIRE CREEK TRIBUTARY NO. 3					
At confluence with Hampshire Creek Tributary No. 2	0.1	9	*	33	*
HAMPSHIRE CREEK TRIBUTARY NO. 4					
At confluence with Hampshire Creek	0.2	24	*	77	*

*Data not available

Table 9 - Summary of Discharges (Continued)

Flooding Source and Location	*Drainage Area (square miles)*	*Peak Discharges (cubic feet per second)* 10-Percent-Annual-Chance	2-Percent-Annual-Chance	1-Percent-Annual-Chance	0.2-Percent-Annual-Chance
INDIAN CREEK					
At Mouth (confluence with Fox River)	14.7	1,095	2,379	3,064	4,511
At confluence with South Tributary	13.9	939	1,966	2,521	3,672
At Austin Avenue	10.7	706	1,472	1,873	2,624
At Scheffer Road	10.5	677	1,378	1,744	2,401
At Farnsworth Avenue	9.5	517	969	1,126	1,527
At Reckinger Road	9.5	505	943	1,097	1,517
At Prairie Path	9.4	491	907	1,056	1,507
At Farnsworth Avenue	7.8	353	560	743	1,290
At Molitor Road	7.6	345	736	814	1,220
At confluence with Selmartin Creek	7.4	345	736	817	1,222
At Interstate 88	4.8	258	539	579	865
At Bilter Road	4.5	298	555	675	965
At Butterfield Road	3.6	231	417	497	797
INDIAN CREEK PRAIRIE PATH RUN					
At Farnsworth Avenue	0.5	46	120	139	146
JERICHO LAKE DIVERSION					
Approximately 1,000 feet downstream of Orchard Road	*	15	*	480	1,190
JELKES CREEK					
At confluence with Fox River	6.8	600	920	1,200	1,760
Just upstream of Boncosky Road	4.7	440	710	930	1,370
Approximately 300 feet downstream of Thorobred Lane	3.2	320	540	700	1,040
At upstream corporate limit of Village of Sleepy Hollow	2.8	280	480	620	930
LAKE RUN					
At confluence with Blackberry Creek (approximately 2680 feet downstream of Hankes Road)	13.0	532	844	1,004	1,419
On Lake Run after the confluence with South of I-88 Diversion (approximately 850 feet east of Route 56 and 2930 feet north of Hankes Road)	*	623	958	1,127	1,544

*Data not available

Table 9 - Summary of Discharges (Continued)

Flooding Source and Location	*Drainage Area (square miles)*	*Peak Discharges (cubic feet per second)* 10-Percent-Annual-Chance	2-Percent-Annual-Chance	1-Percent-Annual-Chance	0.2-Percent-Annual-Chance
LAKE RUN (continued)					
On Lake Run before the confluence with South of I-88 Diversion (approximately 850 feet east of Route 56 and 3710 feet north of Hankes Road)	*	602	732	764	826
Approximately 190 feet upstream of East Bound Illinois Route 56	11.6	672	896	957	1,077
Approximately 1850 feet upstream of Tanner Road	8.9	525	888	1,065	1,527
At confluence with Lake Run Nelson Lake Branch (approximately 780 feet upstream of Seavey Road)	2.9	286	547	689	1,104
At confluence with Lake Run Main Street Branch (approximately 2570 feet downstream of Bliss Road)	1.7	199	369	457	706
LAKE RUN MAIN STREET BRANCH					
At confluence with Lake Run (approximately 3200 feet downstream of Main Street)	2.9	77	162	212	368
Approximately 1310 feet upstream of Main Street	2.3	41	97	133	252
LAKE RUN NELSON LAKE BRANCH					
At confluence with Lake Run (approximately 780 feet upstream of Seavey Road)	5.9	38	54	60	72
LAKE BRANCH NORTH OF I-88 OVERFLOW EAST BRANCH					
On North of I-88 Overflow approximately 1900 feet east of Lake Run and 2100 feet north of I-88 Tollway	*	135	477	686	1,253
On North of I-88 Overflow East Branch approximately 2430 feet east of Lake Run and 2610 feet north of I-88 Tollway	*	33	51	59	78

*Data not available

Table 9 - Summary of Discharges (Continued)

Flooding Source and Location	Drainage Area (square miles)	Peak Discharges (cubic feet per second) 10-Percent-Annual-Chance	2-Percent-Annual-Chance	1-Percent-Annual-Chance	0.2-Percent-Annual-Chance
LAKE BRANCH NORTH OF I-88 OVERFLOW					
On North of I-88 Overflow approximately 1590 feet east of Lake Run and 2670 feet north of I-88 Tollway	*	105	430	632	1,181
LAKE RUN SOUTH OF I-88 DIVERSION					
On South of I-88 Diversion before the confluence with Lake Run (approximately 1850 feet east of Route 56 and 3500 feet north of Hankes Road)	*	20	226	363	718
LORD'S PARK TRIBUTARY[1]					
At the mouth	3.7	*	*	475	*
MAHONEY CREEK					
At confluence with Fox River	2.5	209	344	422	601
MC KEE ROAD TRIBUTARY					
MALGREN DRAIN					
Just Downstream of Malgren Drain / Swan Drain Confluence	2.5	*	*	369	*
MC KEE ROAD TRIBUTARY					
At McKee Street	*	338	554	665	984
At Skyline Drive	4.9	334	546	657	970
At Randall Road	4.2	319	513	618	897
Approximately 3,820 feet upstream of Fabyan Parkway	0.8	157	269	336	527
MILL CREEK					
Approximately 200 feet downstream of abandoned railroad	30.3	1,756	2,987	3,602	5,250
At Kaneville Road	18.3	1,400	*	1,700	*
At State Route 38	8.4	952	*	1,160	*
At Campton Hills Drive	7.3	882	*	1,070	*
At La Fox Road	4.6	700	*	850	*
At State Route 64	3.2	588	*	714	*

*Data not available

[1]Discharges for Lord's Park Tributary were taken from the Cook County FIS. The stream center line resides in Cook County.

Table 9 - Summary of Discharges (Continued)

Flooding Source and Location	Drainage Area (square miles)	Peak Discharges (cubic feet per second) 10-Percent-Annual-Chance	2-Percent-Annual-Chance	1-Percent-Annual-Chance	0.2-Percent-Annual-Chance
MILL CREEK					
TRIBUTARY NO. 2					
At the confluence with Mill Creek Diversion Channel	*	158	*	231	*
NORTH ARM					
BREWSTER CREEK					
At confluence with Brewster Creek	3.4	87	120	131	148
NORTON CREEK					
At confluence with Fox River	11.5	560	849	984	1,325
Approximately 2,400 feet downstream of White Thorn Road	9.8	517	785	909	1,225
Approximately 3,000 feet upstream of White Thorn Road	7.4	438	665	771	1,038
At Dunham Road	4.8	438	665	771	1,038
Upstream of the confluence of Norton Creek Tributary	3.5	365	555	645	870
NORTON CREEK					
TRIBUTARY					
At confluence with Norton Creek	1.8	205	310	360	485
OTTER CREEK					
At confluence with Ferson Creek	33.6	1,061	2,209	2,531	4,853
Above confluence of Otter Creek Tributary	29.7	950	1,973	2,243	4,254
Approximately 1,935 feet downstream from McDonald Road	28.9	937	1,945	2,211	4,194
At McDonald Road	28.2	925	1,922	2,184	4,143
Above confluence of Stony Creek	15.8	526	1,122	1,254	2,362
At Hopps Road	15.3	488	1,025	1,159	2,171
Above confluence of Fitchie Creek	7.2	367	738	812	1,491
At Bowes Road	5.0	306	614	675	1,241
Approximately 4,345 feet upstream from Bowes Road	3.0	195	390	439	760
At Randall Road	1.0	72	145	165	263

*Data not available

Table 9 - Summary of Discharges (Continued)

Flooding Source and Location	*Drainage Area (square miles)*	*Peak Discharges (cubic feet per second)* 10-Percent-Annual-Chance	2-Percent-Annual-Chance	1-Percent-Annual-Chance	0.2-Percent-Annual-Chance
OTTER CREEK WEST					
At confluence with Otter Creek	3.6	211	378	450	916
Approximately 260 feet upstream of Falcon's Trail	2.7	182	327	389	791
Just upstream of unnamed road	2.0	132	267	249	535
PINGREE CREEK					
At mouth	11.1	750	1,213	1,442	1,983
At Highland Avenue	9.5	695	1,119	1,316	1,868
At Soo Line Railroad	9.0	690	1,117	1,301	1,854
At U.S. Route 20	8.7	691	1,139	1,308	1,884
POPLAR CREEK					
At confluence with Fox River	42.3	1,085	1,709	2,010	2,794
PRESTBURY BRANCH					
At confluence with Blackberry Creek (approximately 720 feet downstream of Hankes Road)	2.1	33	57	69	103
At outlet of the upper lake (approximately 1780 feet upstream of Winthrop New Road)	1.8	37	73	93	156
ROUTE 38 BRANCH					
At confluence with Blackberry Creek (approximately 1500 feet downstream of Pouley Road and southeastern to the intersection of Illinois Route 38 and Pouley Road)	0.6	58	92	107	143
SANDY CREEK					
At confluence with Tyler Creek	2.5	320	560	720	1,050
At Randall Road	2.3	225	361	550	829
At U.S. Route 20	0.2	21	46	69	112
SEAVEY ROAD RUN					
Approximately 1650 feet downstream of Illinois Route 47	7.0	364	625	758	1,122
Approximately 920 feet upstream of Interstate 88	5.3	347	587	705	1,020

Table 9 - Summary of Discharges (continued)

Flooding Source and Location	Drainage Area (square miles)	Peak Discharges (cubic feet per second) 10-Percent-Annual-Chance	2-Percent-Annual-Chance	1-Percent-Annual-Chance	0.2-Percent-Annual-Chance
SEAVEY ROAD RUN (continued)					
At confluence of Seavey Road Run and Main Street Branch (approximately 1850 feet upstream of a road to a golf course)	1.9	159	289	357	549
On Seavey Road Run approximately 780 feet upstream of the junction 2820 feet downstream of the bridge on Main Street	*	48	87	108	166
SEAVEY ROAD RUN GREEN ROAD BRANCH					
Approximately 2700 feet downstream (east) of Green Road bridge and approximately 380 feet upstream of the junction with Seavey Road Run	*	95	173	214	329
SEAVEY ROAD RUN MAIN STREET BRANCH					
At confluence of Seavey Road Run and Main Street Branch (approximately 1850 feet upstream of a road to a golf course)	3.5	220	417	522	824
SELMARTEN CREEK					
At Forest Preserve Pond	1.3	45	162	227	438
SLEEPY CREEK					
At confluence with Fox River	2.2	152	338	517	1,039
At Locust Street	1.5	97	259	414	746
At cam approximately 430 feet upstream of Hillcrest Drive	0.9	145	*	351	*
SOUTH TRIBUTARY					
At McClure Road	2.6	133	287	416	883
At Mouth (confluence with Indian Creek)	2.9	277	532	659	984

*Data not available

Table 9 - Summary of Discharges (Continued)

Flooding Source and Location	*Drainage Area (square miles)*	*Peak Discharges (cubic feet per second)* 10-Percent-Annual-Chance	2-Percent-Annual-Chance	1-Percent-Annual-Chance	0.2-Percent-Annual-Chance
STATE STREET CREEK					
At mouth	0.7	123	*	226	*
STATE STREET CREEK TRIBUTARY					
At mouth	0.1	6	*	11	*
STONY CREEK					
At confluence with Otter Creek	11.9	435	885	987	2,007
Above confluence of Bowes Creek	3.3	183	386	434	794
Approximately 5,320 feet above confluence of Bowes Creek	2.9	172	362	407	746
Approximately 2,970 feet downstream of Corron Road	2.8	167	353	397	726
At Corron Road	2.3	138	278	311	558
SUGAR GROVE BRANCH					
Just Downstream of Duffin Drain Confluence	13.2	*	*	1,561	*
At Fay's Lane	4.9	760	1,100	1,260	1,570
Downstream of the confluence with Sugar Grove Branch North	4.3	375	580	680	770
Upstream of the confluence with Sugar Grove Branch North	2.8	180	285	338	435
Downstream of the confluence with Sugar Grove Branch East	2.4	245	415	500	670
Upstream of the confluence with Sugar Grove Branch East	1.1	135	260	310	440
SUGAR GROVE BRANCH EAST					
At confluence with Sugar Grove Branch	1.4	140	180	200	265
SUGAR GROVE BRANCH NORTH					
At confluence with Sugar Grove Branch	1.5	200	285	325	349
TOLLWAY TRIBUTARY	*	*	*	*	*

*Data not available

Table 9 - Summary of Discharges (Continued)

Flooding Source and Location	*Drainage Area (square miles)*	*Peak Discharges (cubic feet per second)* 10-Percent-Annual-Chance	2-Percent-Annual-Chance	1-Percent-Annual-Chance	0.2-Percent-Annual-Chance
TYLER CREEK					
At Randall Road	32.7	1,237	2,073	2,638	3,569
At Big Timber Road	29.1	1,229	1,972	2,448	3,400
At Chicago and Northwestern Railroad	28.7	1,216	1,955	2,409	3,360
At confluence with Pingree Creek	21.1	1,094	1,742	2,096	2,819
At Big Timber Road	10.0	361	548	672	869
At State Route 72	6.2	305	455	547	684
TYLER CREEK UNNAMED TRIBUTARY					
At confluence with Tyler Creek	3.5	88	148	187	*
At Reinking Road	3.0	82	134	162	*
At SOO Railroad	2.5	82	127	153	*
At US Route 20	2.0	115	163	211	*
Approximately 4,200 feet upstream of Route 20	1.3	98	202	281	*
UNION DITCH NO. 2					
At County Line Road	2.9	*	*	356	*
Approximately 1,635 feet upstream of County Line Road	2.1	*	*	287	*
WAUBONSEE CREEK					
Upstream of U.S. Route 30	18.7	774	1,170	1,447	2,700
Downstream of Elgin, Joliet and Eastern Railroad	17.4	734	1,108	1,373	2,500
At Kane-Kendall County boundary	16.5	770	1,220	1,447	1,950
WELCH CREEK					
Just Downstream of Sugar Grove Branch Confluence	36.1	*	*	4,408	*
Just Downstream of Welch Creek Tributary 1 Confluence	21.1	*	*	2,638	*
Just Downstream of Welch Creek Tributary 2 Confluence	18.5	*	*	2,303	*
At Main Street	11.8	*	*	1,407	*
At Rowe South	4.1	*	*	627	*
Just Upstream of Keslinger Road	2.2	*	*	474	*
WEST BRANCH BIG ROCK CREEK					
At US 30	26.6	*	*	3,552	*

*Data not available

Stillwater elevations have been determined for the 10-, 2-, 1-, and 0.2-percent-annual-chance floods for the flooding sources studied by detailed methods and are summarized in Table 10, "Summary of Stillwater Elevations."

Table 10 - Summary of Stillwater Elevations

Flooding Source and Location	*Elevation (feet NAVD88)*			
	10-Percent-Annual-Chance	*2-Percent-Annual-Chance*	*1-Percent-Annual-Chance*	*0.2-Percent-Annual-Chance*
CITY OF AURORA/ UNINCORPORATED KANE COUNTY				
Mastodon Lake	659.38	*	661.29	662.99

*Data not available

3.2 Hydraulic Analyses

Analyses of the hydraulic characteristics of flooding from the sources studied were carried out to provide estimates of the elevations of floods of the selected recurrence intervals. Users should be aware that flood elevations shown on the FIRM represent rounded whole-foot elevations and may not exactly reflect the elevations shown on the Flood Profiles or in the Floodway Data tables in the FIS report. Flood elevations shown on the FIRM are primarily intended for flood insurance rating purposes. For construction and/or floodplain management purposes, users are encouraged to use the flood elevation data presented in this FIS in conjunction with the data shown on the FIRM.

Pre-countywide FIS

Each incorporated community within, and the unincorporated areas of, Kane County, with the exceptions of the villages of Barrington Hills, Big Rock, Burlington, Campton Hills, Elburn, Gilberts, Hoffman Estates, Kaneville, Lily Lake, Pingree Grove, and Virgil, has a previously printed FIS report. The hydraulic analyses described in those reports that have not been superseded by new study information are summarized below.

Within the unincorporated areas of Kane, water-surface elevations for floods of the selected recurrence intervals for Bowes Creek, Bowes Creek South, Brewster Creek, Ferson Creek, Fitchie Creek, Mahoney Creek, North Arm Brewster Creek, Otter Creek, and Otter Creek West were computed using the USACE HEC-2 step-backwater computer program (Reference 74).

The water-surface elevations for Poplar Creek were obtained from the floodplain information report for Poplar Creek watershed in which elevations were determined using the WSP-2 program (Reference 75, 76).

The water-surface elevations for Mill Creek were determined by the slope/area method and a rating curve from a cross section located 2,714 feet downstream of Kaneville Road.

Starting water-surface elevations were calculated using corresponding flood elevations on the main stem, flood profiles from previous studies by the State of Illinois for Brewster Creek, Ferson Creek, Hampshire Creek South, Mahoney Creek, McKee Road Tributary, Mill Creek, and North Arm Brewster Creek, and rating curves (Reference 4, 77).

The water-surface elevations on the Fox River, the Fox River East Channel, and Waubonsee Creek were computed using the USACE HEC-2 step-backwater program (Reference 74). Cross sections and structural data for the Fox River and the Fox River East Channel were provided by the IDOT-DWR from field surveys (Reference 78, 79). Cross sections and structural data for Waubonsee Creek were obtained from the Illinois State Water Survey (Reference 80). Cross sections for the backwater analyses were located at close intervals above and below bridges and culverts in order to compute the significant backwater effects from these structures.

The only serious backwater effect due to bridge constriction is on Waubonsee Creek. Backwater effects from Montgomery Dam have become a problem according to residents in the area. This situation was also studied.

In the unincorporated areas of Kane, starting water-surface elevations on the Fox River, the Fox River East Channel, and Waubonsee Creek were based on the slope/area method. Starting elevations for the 10-, 2-, 1-, and 0.2-percent-annual-chance floods for the Fox River, in other communities, were based upon the discharge recurrence interval rating curves at Carpentersville Dam, Elgin Dam, Geneva Dam, and South Elgin Dam (Reference 81).

In Montgomery, water-surface profiles for Fox River Tributary and Fox River Tributary (East Branch) were determined for the 10-, 2-, 1-, and 0.2-percent-annual-chance floods by use of the USACE HEC-2 computer program. Starting water-surface elevations were determined with either known high-water marks, by assuming critical depth, or by the slope/area method.

Water-surface elevation for floods of the selected recurrence intervals of Four Winds Way Creek and Carpenter Creek were computed through use of the USACE HEC-2 step-backwater computer program (Reference 74). This program relates stream geometry, characteristics, and discharge to stream elevation. Flood profiles were drawn showing computed water-surface elevations to an accuracy of 0.5 foot for floods of selected recurrence intervals.

Starting water-surface elevations for Carpenter Creek and Four Winds Way Creek were determined using normal depth analysis. Flood elevations can often be increased by ice jams during spring thaws or by debris clogging bridges.

Water-surface elevations of floods of the selected recurrence intervals of Geneva Creek were computed through use of the USACE HEC-2 backwater computer program (Reference 74). The starting water-surface elevation for Geneva Creek

was determined using the normal depth subroutine of the USACE HEC-2 computer model (Reference 74).

Cross sections for Hampshire Creek South were obtained from field surveys, except for some overbank portion cross sections that were determined using topographic maps (Reference 82). Cross sections for Hampshire Creek and Hampshire Creek Tributary No. 1 were obtained from field surveys performed by Engineering Enterprises, Inc. and aerial photographs (Reference 83, 84).

Water-surface elevations of floods of the selected recurrence intervals on Hampshire Creek South were computed using the USACE HEC-2 step-backwater computer program (Reference 74). For Hampshire Creek and Hampshire Creek Tributary No. 1, water-surface elevations of floods of the selected recurrence intervals were computed using the WSP-2 step-backwater computer program (Reference 76). Starting water-surface elevations for Hampshire Creek South and Hampshire Creek were calculated using the slope/area method. Starting water-surface elevations for Hampshire Creek Tributary No. 1 were calculated using the slope/area method, assuming non-coincident timing of peaks.

All data used in the hydraulic analyses for Union Ditch No. 2 were obtained from records of past floods and through engineering judgment. In cases where the flood elevations are close together on the flood profile, due to the limitations of the profile scale, only the 1-percent-annual-chance profile has been drawn.

Starting water-surface elevations for the 10-, 2-, 1-, and 0.2-percent-annual-chance floods for Jelkes Creek and the starting elevations for the 10- and 1-percent-annual-chance floods for Sleepy Creek were based upon the normal depth method.

Water-surface elevations of floods of the selected recurrence intervals of Jelkes Creek and Sleepy Creek were computed through use of the USACE HEC-2 step-backwater computer program (Reference 74). The hydraulic analysis for Sleepy Creek was made in two segments: between the mouth and Hillcrest Drive and between Hillcrest Drive and Illinois Route 72. This was done to correctly model a dam break at river station 11,000 at the dam located approximately 430 feet upstream of Hillcrest Drive. An analysis of the hydraulics indicated that a major flood would overtop the dam and result in failure.

The starting water-surface elevation for 7th Avenue Creek and 7th Avenue Creek Tributary were determined using the normal depth subroutine of the USACE HEC-2 computer model (Reference 74).

Water-surface elevations of floods of the selected recurrence intervals for Jericho Lake Diversion were computed using the USACE HEC-2 step-backwater computer program (Reference 74).

The water-surface elevations for floods of the selected recurrence intervals of 7th Avenue Creek and 7th Avenue Creek Tributary were computed through use of the USACE HEC-2 step-backwater computer program (Reference 74). Water-surface elevations for the floods of selected recurrence intervals of State Street Creek and State Street Creek Tributary were computed assuming normal depth at survey cross sections. Flood profiles were drawn showing computed water-surface elevations to an accuracy of 0.5 foot for floods of selected recurrence intervals. Flood elevations

can often be increased by ice jams during spring thaws or by debris clogging bridges. It should be noted that roughness values were increased during model calibration to reflect scour on 7th Avenue Creek and 7th Avenue Creek Tributary.

Cross sections for the backwater analyses on Sugar Grove Branch and Sugar Grove Branch East (referenced as Welch Creek and Welch Creek Tributary No. 1 at the time of study) were determined from field surveys. Water-surface elevations of floods of the selected recurrence intervals were computed through use of the USACE HEC-2 step-backwater computer program (Reference 74). The starting water-surface elevations used for Sugar Grove Branch and Sugar Grove Branch East were calculated using the slope/area method. (Please note that this information applies only to the area south of the railroad. The detailed study of Sugar Grove Branch upstream of the railroad was superseded as a result of LOMR 07-05-0178P. See "August 3, 2009 Revised Countywide FIS" below.)

The Hydrological Investigation Atlas for the Sugar Grove quadrangle accurately depicts past historical flooding (Reference 85). It was judged a satisfactory source for assessing the flood potential in those areas not studied by detailed methods in this report.

The starting downstream water-surface elevations used in the HEC-2 step-backwater program were computed by the normal depth methods for Norton Creek and Norton Creek Tributary (Reference 74). Water-surface elevations of floods of the selected recurrence intervals on Norton Creek and Norton Creek Tributary were computed through use of the USACE HEC-2 step-backwater computer program (Reference 74).

For Lord's Park Tributary in Elgin, the WSP-2 computer program was used to determine water-surface elevations of floods of the selected recurrence intervals. Starting water-surface elevations were computed by the normal depth methods.

December 20, 2002
Initial Countywide FIS

The HEC-2 model of Tyler Creek was extended from Big Timber Road to Illinois Route 72. The new Randall Road Bridge was incorporated into the model. Additional cross sections were added downstream of Big Timber Road based on surveys prepared by the City of Elgin. The HEC-2 model of Sandy Creek was extended from Randall Road to U.S. Route 20. Pingree Creek was modeled using HEC-2 from its confluence with Tyler Creek to U.S. Route 20. The Tyler Creek floodway was redefined from Randall Road to Big Timber Road based on the latest State of Illinois criteria. Tyler Creek upstream of Big Timber Road, Sandy Creek upstream of Randall Road, and Pingree Creek floodways were defined according to the State of Illinois regulatory floodway criteria.

Cross sections for the backwater analyses for Eakin Creek were determined by field surveys, with some overbank sections determined from topographic maps. Cross section locations were at close intervals above and below bridges, dams, and culverts in order to compute the significant backwater effects of these structures. All bridges were surveyed to determine structural geometry. The 10-,

2-, 1-, and 0.2-percent-annual-chance recurrence intervals for Eakin Creek were studied using the USACE HEC-RAS model (Reference 86).

November 16, 2006
Revised Countywide FIS

No new or revised hydraulic studies were incorporated into the November 16, 2006 revised countywide FIS.

August 3, 2009
Revised Countywide FIS

The hydraulic analysis for the streams located within the Blackberry Creek watershed (Aurora Chain of Lakes, Aurora Chain of Lakes Cherry Hills Diversion, Blackberry Creek, East Run, East Run North Branch, East Run North Loop, Elburn Run, Lake Run, Lake Run Main Street Branch, Lake Run Nelson Lake Branch, Lake Run North of I-88 Overflow, Lake Run North of I-88 Overflow East Branch, Lake Run South of I-88 Diversion, Prestbury Branch, Route 38 Branch, Seavey Road Run, Seavey Road Run Green Road Branch, Seavey Road Run Main Street Branch) were prepared using the HEC-RAS hydraulic model (Reference 87). The HEC-RAS analysis was used to route the flood-peak discharge and determine the flood elevations throughout Blackberry Creek watershed.

The two-dimensional, finite-element, surface-water-modeling system (FESWMS) (Reference 88) was used for analyzing the flow diversion at Jericho Lake near Montgomery, Illinois. Results from the FESWMS model have been applied to determine the amount of discharge being diverted out of Blackberry Creek watershed through the lake. These results were used in the routing functions of the hydrologic model (Reference 48).

Cross sections from the WSP-2 hydraulic routing model developed by the U.S. Department of Agriculture, Soil Conservation Service study in 1985 (Reference 89) were used for the analysis of Blackberry Creek. Data for bridges and culverts constructed since the 1985 study were surveyed by the IDNR-OWR, Smith Engineering Consultants, Inc., and the USGS. The hydraulic model was calibrated and verified using high water marks and observed inundation maps for the July 17-18, 1996 flood event (Reference 48).

The hydraulic analysis for the Indian Creek watershed (Indian Creek, Indian Creek Prairie Path Run, Selmarten Creek, South Tributary, and Tollway Tributary) was modeled in two separate sections. For the northern portion of the watershed, the FEQ unsteady flow program was used. For the southern portion, the steady-state processor within HEC-RAS was used.

LOMR 07-05-0178P should be referenced for hydraulic information for Sugar Grove Branch, Sugar Grove Branch East, and Sugar Grove Branch North (referenced in the LOMR as Welch Creek, Welch Creek Tributary No. 1, and

Welch Creek Tributary No. 2). The portion of the pre-countywide Sugar Grove Branch study area upstream of the railroad was superseded by this LOMR.

To be determined
Revised Countywide FIS

For this PMR, the hydraulic analysis for Big Rock and Welch Creek watershed in Kane County was completed by the Illinois State Water Survey for Kane County in December 2008. The watershed was divided into two HEC-RAS version 4.0 models (Reference 90) - one for Big Rock Creek and its tributaries and one for Welch Creek and its tributaries. Detailed study with limited survey was completed for the following streams (limits given in Table 4) within Kane County: Welch Creek, Sugar Grove Branch (downstream of the existing detailed study), Big Rock Creek, West Branch Big Rock Creek, East Branch Big Rock Creek, and Malgren Drain.

Digital elevation data available from Kane County were used to generate cross section data input for the model. Where available, as-built bridge plans were reviewed and used to model these structures. Where bridge plans were not available, field measurements and survey data were collected. Photos were taken throughout the watershed to document existing conditions and determine roughness coefficients for modeling.

The models were calibrated to the September 12-14, 2008 flood event using stage gage data and high water observations. Peak discharges calculated using HEC-HMS were input to the RAS model, and water surface elevations and subsequent extent of flooding simulated by the model were compared with observations and information recorded at the stage gages. Only the 1-percent- annual-chance flood profile was determined as part of this study.

Approximate studies were completed for the remaining stream reaches in these watersheds within Kane County (see Table 6).

The hydraulic analyses for this study were based on unobstructed flow. The flood elevations shown on the Flood Profiles (Exhibit 1) are thus considered valid only if hydraulic structures remain unobstructed, operate properly, and do not fail.

Locations of selected cross sections used in the hydraulic analyses are shown on the flood profiles (Exhibit 1). For stream segments for which a floodway was computed (Section 4.2), selected cross section locations are also shown on the FIRM (Exhibit 2).

Effective flow areas of the floodplain, cross sections, loss coefficients, and overbank roughness coefficients (Manning's "n") were assigned to each cross section based on field inspection. The range of the Manning's "n" coefficients for each stream is shown in Table 11, "Roughness Coefficients (Manning's 'n' Values)."

Table 11 - Roughness Coefficients (Manning's "n" Values)

Stream	Channel "n"	Overbank "n"
7th Avenue Creek	0.025-0.10	0.05-0.08
7th Avenue Creek Tributary	0.05-0.06	0.08
Anderson Road Run	*	*
Anderson Road Run North Branch	*	*
Aurora Chain of Lakes	0.015-0.055	0.02-0.10
Aurora Chain of Lakes Cherry Hills Diversion	0.015-0.055	0.045-0.10
Big Rock Creek	0.02-0.045	0.01-0.11
Blackberry Creek	0.040-0.070	0.06-0.13
Bowes Creek	0.035-0.072	0.065-0.100
Bowes Creek South	0.045	0.070
Brewster Creek	0.060	0.140
Carpenter Creek	0.10	0.12
Duffin Drain	0.02-0.045	0.01-0.11
Eakin Creek	0.05	0.06
East Branch Big Rock Creek	0.02-0.045	0.01-0.11
East Branch Big Rock Creek Tributary 2	0.02-0.045	0.01-0.11
East Run	0.040-0.080	0.045-0.115
East Run North Branch	0.045	0.105
East Run North Loop	0.045	0.105
Elburn Run	0.04-0.055	0.065-0.115
Ferson Creek	0.030-0.08	0.070-0.100
Fitchie Creek	0.035-0.104	0.070-0.100
Four Winds Way Creek	0.10	0.12
Fox River	0.025-0.10	0.035-0.10
Fox River East Channel	0.025-0.040	0.060-0.070
Fox River Tributary	0.020-0.060	0.050-0.080
Fox River Tributary East Branch	0.040	0.050
Geneva Creek	0.040-0.085	0.04-0.10
Hampshire Creek	0.046-0.150	0.035-0.145
Hampshire Creek South	0.030-0.100	0.030-0.080
Hampshire Creek Tributary No. 1	0.050-0.120	0.050

*Data not available

Table 11 - Roughness Coefficients (Manning's "n" Values) (Continued)

Stream	Channel "n"	Overbank "n"
Hampshire Creek Tributary No. 2	0.035-0.110	0.040-0.090
Hampshire Creek Tributary No. 3	0.065-0.075	0.050
Hampshire Creek Tributary No. 4	0.075-0.080	*
Indian Creek	0.015-0.060	0.020-0.120
Indian Creek Prairie Path Run	*	*
Jelkes Creek	0.035-0.070	0.050-0.090
Jericho Lake Diversion	0.050-0.055	0.065-0.085
Lake Run	0.045-0.075	0.055-0.125
Lake Run Nelson Lake Branch	0.045-0.075	0.055-0.125
Lake Run North of I-88 Overflow East Branch	0.045-0.075	0.055-0.125
Lake Run South of I-88 Diversion	0.045-0.075	0.055-0.125
Lake Run Main St. Branch	0.045-0.075	0.055-0.125
Lake Run North of I-88 Overflow	0.045-0.075	0.055-0.125
Lord’s Park Tributary	0.060-0.080	0.015-0.040
Mahoney Creek	0.100	0.120
Malgren Drain	0.02-0.045	0.01-0.11
McKee Road Tributary	0.035-0.090	0.050-0.100
Mill Creek	0.020-0.100	0.040-0.140
Mill Creek Tributary 2	0.060	0.080-0.100
North Arm Brewster Creek	0.080	0.120
Norton Creek	0.045-0.15	0.05-0.150
Norton Creek Tributary	0.030-0.055	0.045-0.055
Otter Creek	0.035-0.075	0.070-0.090
Otter Creek West	0.035-0.055	0.070-0.085
Pingree Creek	0.055-0.065	0.05-0.08
Poplar Creek	0.015-0.040	0.060-0.080
Prestbury Branch	0.045-0.06	0.115-0.165
Route 38 Branch	0.04-0.06	0.085-0.105
Sandy Creek	0.04-0.09	0.07-0.12
Seavey Road Run	0.05-0.07	0.09-0.12

*Data not available

Table 11 - Roughness Coefficients (Manning's "n" Values) (Continued)

Stream	Channel "n"	Overbank "n"
Seavey Road Run Green Road Branch	0.05-0.063	0.08-0.12
Seavey Road Run Main St. Branch	0.045-0.06	0.105-0.125
Selmarten Creek	0.015-0.059	0.030-0.100
Sleepy Creek	0.05-0.10	0.1
South Tributary	0.035-0.060	0.060-0.150
State Street Creek	*	*
State Street Creek Tributary	*	*
Stony Creek	0.030-0.072	0.060-0.110
Sugar Grove Branch (downstream of profile station 3,565)	0.02-0.045	0.01-0.11
Sugar Grove Branch (upstream of profile station 3,565)	0.045-0.090	0.050-0.090
Sugar Grove Branch East	0.045-0.090	0.050-0.090
Sugar Grove Branch North	*	*
Tollway Tributary	*	*
Tyler Creek	0.045-0.07	0.05-0.1
Tyler Creek Unnamed Tributary	*	*
Union Ditch No. 2	*	*
Waubonsee Creek	0.035-0.055	0.050-0.070
Welch Creek	0.02-0.045	0.01-0.11
West Branch Big Rock Creek	0.02-0.045	0.01- 0.11

*Data not available

3.3 Vertical Datum

All FISs and FIRMs are referenced to a specific vertical datum. The vertical datum provides a starting point against which flood, ground, and structure elevations can be referenced and compared. Until recently, the standard vertical datum in use for newly created or revised FISs and FIRMs was the National Geodetic Vertical Datum of 1929 (NGVD 29). With the finalization of the North American Vertical Datum of 1988 (NAVD 88), many FIS and FIRMs are being prepared using NAVD 88 as the referenced vertical datum.

All flood elevations shown in this FIS and on the FIRM are referenced to NAVD 88. Structure and ground elevations in the community must, therefore, be referenced to NAVD 88. It is important to note that adjacent counties may be referenced to NGVD 29. This may result in differences in base flood elevations (BFEs) across the county boundary.

For more information on NAVD 88, see *Guidelines and Specifications for Flood Hazard Mapping Partners Appendix B: Guidance for Converting to the North American Vertical Datum of 1988* (Reference 91) available at http://www.fema.gov/plan/prevent/fhm/dl_cgs.shtm or contact the Vertical Network Branch, National Geodetic Survey, Coast and Geodetic Survey, National Oceanic and Atmospheric Administration, Rockville, Maryland 20910 (Internet address http://www.ngs.noaa.gov).

Temporary vertical monuments are often established during the preparation of a flood hazard analysis for the purpose of establishing local vertical control. Although these monuments are not shown on the FIRM, they may be found in the Technical Support Data Notebook associated with the FIS report and FIRM for this county. Interested individuals may contact FEMA to access these data.

August 3, 2009 FIS

Information for the August 3, 2009 FIS was converted from NGVD 29 to NAVD 88 based on data presented in Figure 1 and Table 12a. Computations show an average conversion factor of -0.206 feet (NGVD 29 – 0.206 = NAVD 88) for the county. The Single Conversion Factor (countywide) method was applied uniformly across the county, except as noted below, and used to prepare the Summary of Stillwater Elevations Table, Floodway Data Tables, Flood Profiles, and FIRMs.

The Multiple Conversion Factors (stream-by-stream) method was implemented for a stream when a detailed study reach was located in two or more counties (multi-county stream) and the countywide conversion factor for each county differed by more than one-tenth of a foot. For the stream-by-stream method, the stream is assigned an average conversion factor based on the conversion factors computed at three points along the stream. These results are shown in Table 12b.

For this Revision

Study information for this PMR is referenced to NAVD 88 and required no conversion.

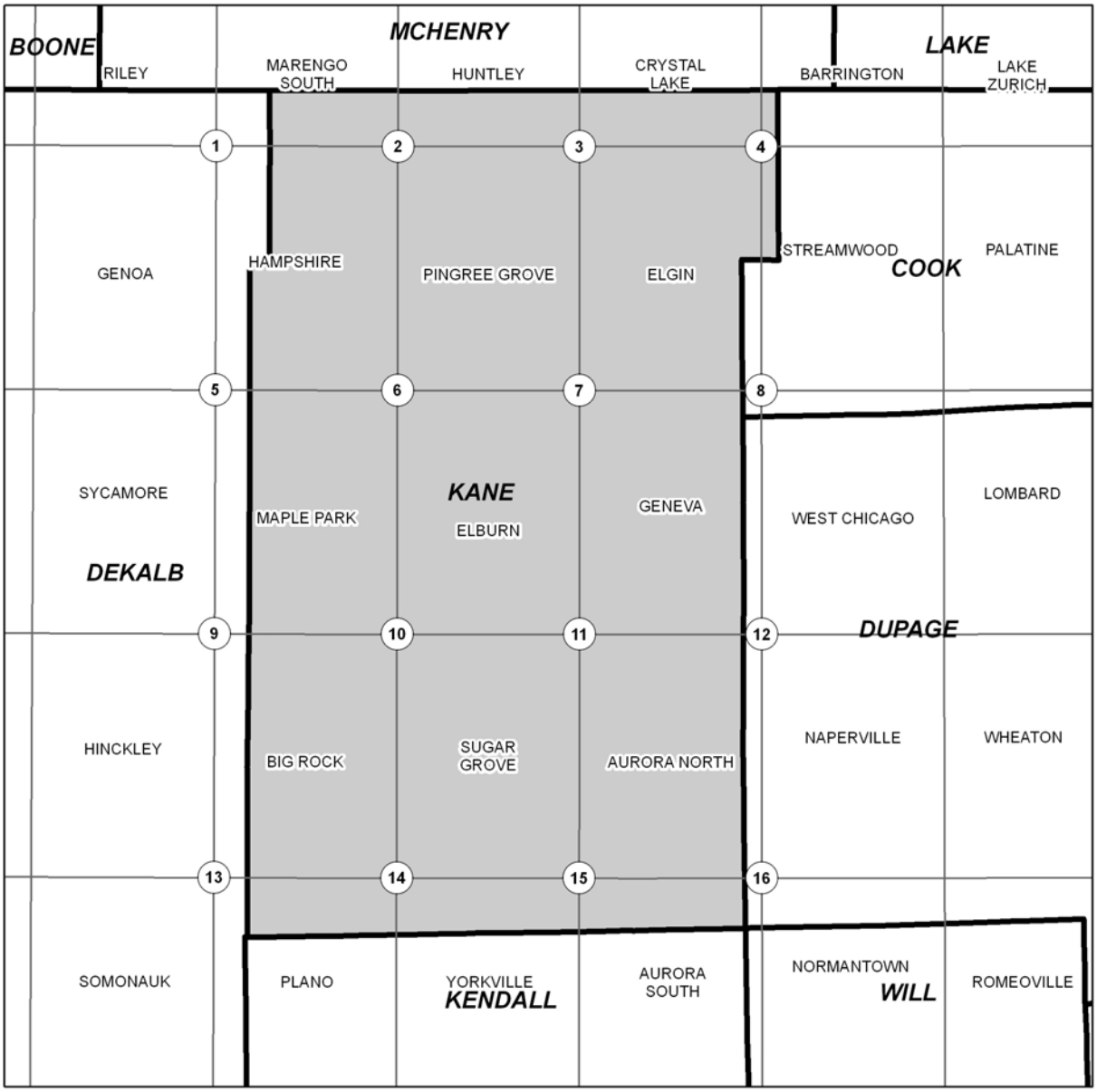

Figure 1 – Vertical Datum Conversion
USGS Quadrangle Corner Intersections
The change in elevation for each Point ID is listed in Table 12a.

Table 12a - Vertical Datum Conversions
Single Conversion Factor (countywide) Method
Kane County

Point ID #	Quadrangle Name	Corner	NAD83 Latitude (dec. deg.)	NAD83 Longitude (dec. deg.)	NGVD29 to NAVD88 Elevation Change (feet)
1	Hampshire	NW	42.125	88.625	-0.174
2	Pingree Grove	NW	42.125	88.500	-0.174
3	Elgin	NW	42.125	88.375	-0.190
4	Streamwood	NW	42.125	88.250	-0.203
5	Maple Park	NW	42.000	88.625	-0.154
6	Elburn	NW	42.000	88.500	-0.177
7	Geneva	NW	42.000	88.375	-0.226
8	West Chicago	NW	42.000	88.250	-0.262
9	Big Rock	NW	41.875	88.625	-0.171
10	Sugar Grove	NW	41.875	88.500	-0.197
11	Aurora North	NW	41.875	88.375	-0.226
12	Naperville	NW	41.875	88.250	-0.240
13	Plano	NW	41.750	88.625	-0.203
14	Yorkville	NW	41.750	88.500	-0.226
15	Aurora South	NW	41.750	88.375	-0.233
16	Normantown	NW	41.750	88.250	-0.243

Range of conversion values	-0.262 through -0.154
Average conversion factor	**-0.206**
Maximum variance from the average conversion	0.056
Maximum variance from a no-conversion value	-0.052

Table 12b - Vertical Datum Conversions
Multiple Conversion Factors (Stream by Stream) Method

Point Location	County	NAD83 Latitude (dec. deg.)	NAD83 Longitude (dec. deg.)	NGVD29 to NAVD88 Elevation Change (Feet)	Maximum Offset	Average Conversion
LORD'S PARK TRIBUTARY						
Downstream	Cook	42.023	88.258	-0.256		
Intermediate	Cook	42.029	88.262	-0.256		
Upstream	Cook	42.036	88.261	-0.253	-0.002	-0.255
NORTON CREEK						
Downstream	Kane	47.949	88.311	-0.243		
Intermediate	Kane	41.947	88.280	-0.246		
Upstream	DuPage	41.938	88.249	-0.249	-0.003	-0.246
NORTON CREEK TRIBUTARY						
Downstream	Kane	41.948	88.264	-0.249		
Intermediate	DuPage	41.954	88.253	-0.253		
Upstream	DuPage	41.958	88.242	-0.253	0.003	-0.251
POPLAR CREEK						
Downstream	Kane	42.013	88.278	-0.256		
Intermediate	Cook	42.044	88.165	-0.256		
Upstream	Cook	42.110	88.166	-0.223	0.022	-0.245
WAUBONSEE CREEK						
Downstream	Kendall	41.686	88.354	-0.233		
Intermediate	Kendall	41.722	88.298	-0.233		
Upstream	DuPage	41.752	88.232	-0.246	-0.013	-0.237

4.0 FLOODPLAIN MANAGEMENT APPLICATIONS

The NFIP encourages state and local governments to adopt sound floodplain management programs. Therefore, each FIS provides 1-percent-annual-chance flood elevations and delineations of the 1- and 0.2-percent-annual-chance floodplain boundaries and 1-percent-annual-chance floodway to assist communities in developing floodplain management measures. This information is presented on the FIRM and in many components of the FIS report, including Flood Profiles, Floodway Data tables, and Summary of Stillwater Elevation tables. Users should reference the data presented in the FIS as well as additional information that may be available at the local community map repository before making flood elevation and/or floodplain boundary determinations.

4.1 Floodplain Boundaries

To provide a national standard without regional discrimination, the 1-percent-annual-chance flood has been adopted by FEMA as the base flood for floodplain management purposes. The 0.2-percent-annual-chance flood is employed to indicate additional areas of flood risk in the community. For the flooding sources studied by detailed methods, the 1- and 0.2-percent-annual-chance floodplain boundaries have been delineated using the flood elevations determined at each cross section. Between cross sections, the boundaries were interpolated on the basis of available topography.

August 3, 2009 Revision

Between cross sections, the floodplain boundaries for streams studied by detailed methods were re-delineated using the 2004 countywide 2-foot contour dataset prepared using 2001 aerial photography and provided by Kane County (Reference 37).

The floodplain boundaries for revised detailed studied streams were delineated on the basis of available topography. The streams included Indian Creek watershed (Indian Creek, Indian Creek Prairie Path Run, Selmarten Creek, South Tributary and Tollway Tributary) and Blackberry Creek watershed (Aurora Chain of Lakes, Aurora Chain of Lakes Cherry Hills Diversion, Blackberry Creek, East Run, East Run North Branch, East Run North Loop, Elburn Run, Lake Run, Lake Run Main Street Branch, Lake Run Nelson Lake Branch, Lake Run North of I-88 Overflow, Lake Run North of I-88 Overflow East Branch, Lake Run South of I-88 Diversion, Prestbury Branch, Route 38 Branch, Seavey Road Run, Seavey Road Run Green Road Branch, Seavey Road Run Main Street Branch).

The 1- and 0.2-percent-annual-chance floodplain boundaries were provided by the USGS for the streams within the Blackberry Creek watershed. The USGS used the 2004 Kane County topographic data from 2001 aerial photography to delineate the floodplain boundaries. The 1-percent-annual-chance floodplain boundaries for the streams in the Indian Creek watershed were provided by V_3 Companies, Ltd., and were delineated by V_3, on the basis of the 1986 Kane County topographic data. Floodplain boundaries were revised where necessary to match the 2004 Kane

County topographic data. The 0.2-percent-annual-chance floodplain was manually delineated by the ISWS using the 2004 Kane County topographic data.

For this Revision

For this PMR, between cross sections the 1-percent-annual-chance floodplain boundaries for the streams studied by detailed methods with limited survey were delineated using the Kane County topographic data prepared using aerial photography obtained during spring 2001, which have 2-foot contour intervals (Reference 37).

The 1- and 0.2-percent floodplain boundaries are shown on the FIRM (Exhibit 2). On this map, the 1-percent-annual-chance floodplain boundary corresponds to the boundary of the areas of special flood hazards (Zones A, AH, AO, and AE); and the 0.2-percent-annual-chance floodplain boundary corresponds to the boundary of areas of moderate flood hazards. In cases where the 1- and 0.2-percent-annual-chance floodplain boundaries are close together or collinear, only the 1-percent-annual-chance floodplain boundary has been shown. Small areas within the floodplain boundaries may lie above the flood elevations but cannot be shown due to limitations of the map scale and/or lack of detailed topographic data.

For the streams studied by approximate methods, only the 1-percent-annual-chance floodplain boundary is shown on the FIRM (Exhibit 2). The boundaries of the 1-percent-annual-chance floodplains in approximate zones were delineated using the previously printed FIRMs for all of the incorporated and unincorporated areas within Kane County.

4.2 Floodways

Encroachment on floodplains, such as structures and fill, has the potential to reduce flood-carrying capacity, increase flood heights and velocities, and increase flood hazards in areas beyond the encroachment itself. For purposes of the NFIP, a floodway is used as a tool to assist local communities in this aspect of floodplain management. Under this concept, the area of the 1-percent-annual-chance floodplain is divided into a floodway and a flood fringe.

The floodway is the channel of a stream, plus any adjacent floodplain areas (see Figure 2, “Floodway Schematic”) that must be kept free of encroachment so that the 1-percent-annual-chance flood can be carried without substantial increases in flood heights. Minimum federal standards limit such increases to 1.0 foot, provided that hazardous velocities are not produced. In Illinois however, under the *Rivers, Lakes and Streams Act* (615 ILCS 5/23, 29 & 30 and 615 ILCS 5/18), encroachment in the floodplain is limited to that which will cause only an insignificant increase in flood heights (Reference 92). The State of Illinois has adopted this more stringent criterion which limits the increase in flood heights to 0.1 foot, no more than a 10 percent reduction in floodplain volume, and no more than a 10 percent increase in average velocity. This has generally been interpreted as the least surcharge measurable, consistent with the encroachment option of the computer program utilized for the floodway determination. The floodways in this

FIS are presented to local agencies as a minimum standard that can be adopted directly or that can be used as a basis for additional floodway studies.

Floodways in northeastern Illinois are further defined in Part 3708 of the *Rivers, Lakes and Streams Act.* Areas included in these regulations are Cook, DuPage, Kane, Lake, McHenry, and Will Counties, except for those areas which are within Chicago. Section 3708.60 (c) contains the floodway definition for northeastern Illinois as follows:

> The regulatory floodway boundaries are determined by hydraulic and hydrologic analyses, which calculate that portion of the floodplain which must be preserved to store and discharge floodwaters without causing damaging or potentially damaging increases in flood stages and flood velocities or loss of flood storage which would result in singularly or cumulatively in more than a 0.1 foot increase in flood stage or a 10-percent increase in velocity.

This is commonly called the "storage floodway," whereas the typical floodway is commonly called the "conveyance-only floodway." These rules allow for communities to adopt a "conveyance-only floodway" instead of a "storage floodway" according to Section 3608.60 (d):

> The need to preserve storage when defining the regulatory floodway will be waived by the Department if all of the municipalities and counties along a hydraulically significant portion of the watershed require effective compensatory storage for all construction and fill in the 100-year frequency floodplain. Effective compensatory storage requires floodplain storage volumes to be replaced at the same flood frequency event as previously existed. Additionally, legal assurances such as easements must be provided so that the compensatory site will remain open to the stream in order to allow floodwater to reach it.

The floodway presented in this FIS report and on the FIRM was computed for certain stream segments on the basis of equal conveyance reduction from each side of the floodplain. Floodway widths were computed at cross sections. Between cross sections, the floodway boundaries were interpolated. The results of the floodway computations have been tabulated for selected cross sections (Table 13, "Floodway Data"). The computed floodways are shown on the FIRM (Exhibit 2). In cases where the floodway and 1-percent-annual-chance floodplain boundaries are either close together or collinear, only the floodway boundary is shown.

The area between the floodway and 1-percent-annual-chance floodplain boundaries is termed the flood fringe. The flood fringe encompasses the portion of the floodplain that could be completely obstructed without increasing the water-surface elevation of the 1-percent-annual-chance flood by more than 0.1 foot at any point. Typical relationships between the floodway and the flood fringe and their significance to floodplain development are shown in Figure 2, "Floodway Schematic."

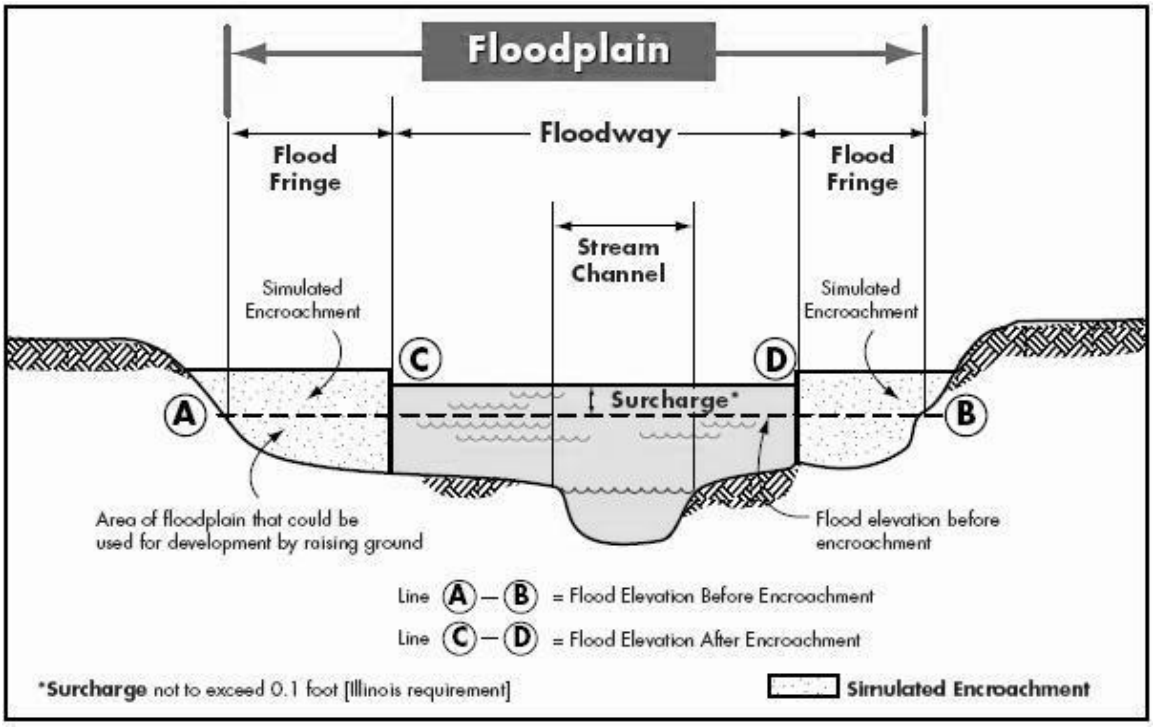

Figure 2 – Floodway Schematic

August 3, 2009 Revision

Floodways were established for Indian Creek, Selmarten Creek and South Tributary. Floodways were not established for Indian Creek Prairie Path Run and Tollway Tributary because their drainage areas do not exceed the one square mile limit established by the IDNR-OWR. Within the Blackberry Creek watershed, floodways were defined for the following streams: Aurora Chain of Lakes, Aurora Chain of Lakes Cherry Hills Diversion, Blackberry Creek, East Run, East Run North Branch, East Run North Loop, Elburn Run, Lake Run, Lake Run Main Street Branch, Lake Run Nelson Lake Branch, Lake Run South of I-88 Diversion, Prestbury Branch, Route 38 Branch, Seavey Road Run, Seavey Road Run Green Road Branch, and Seavey Road Run Main Street Branch. Floodways were not defined for Lake Run North of I-88 Overflow and Lake Run North of I-88 Overflow East Branch because their drainage areas do not exceed the one square mile limit established by the IDNR-OWR.

Near the mouths of streams studied in detail, floodway computations are made without regard to flood elevations on the receiving water body. Therefore, "Without Floodway" elevations presented in Table 13, "Floodway Data" for certain downstream cross sections of Jelkes Creek, Hampshire Creek South, Bowes Creek, Sandy Creek, North Arm Brewster Creek, and Brewster Creek are lower than the regulatory flood elevations in that area, which must take into account the 1-percent-annual-chance flooding due to backwater from other sources.

For this Revision

No floodways were added or revised as a result of this PMR.

In the State of Illinois, any portion of a stream or watercourse that lies within the floodway fringe of a studied (AE) stream may have a state regulated floodway. The FIRM may not depict these state regulated floodways.

Floodways restricted by anthropogenic features such as bridges and culverts are drawn to reflect natural conditions and may not agree with the widths listed in the floodway data table in the Flood Insurance Study. The floodway as shown on the FIRM should be used for regulatory purposes.

Encroachment into areas subject to inundation by floodwaters having hazardous velocities aggravates the risk of flood damage, and heightens potential flood hazards by further increasing velocities. In order to reduce the risk of property damage in areas where the stream velocities are high, the community may wish to restrict development in areas outside the floodway.

In Illinois, along streams where floodways have not been computed, the community must obtain state permit approval (when applicable) for development. This ensures that the cumulative effect of development in the floodplain will not cause an increase in the base flood elevations that creates a potential for flood damages.

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
7th Avenue Creek								
A	623[1]	18	37	8.2	682.2	680.5[4]	680.5	0.0
B	2,878[1]	31	94	2.9	704.3	704.3	704.4	0.1
C	5,232[1]	28	132	1.9	711.7	711.7	711.7	0.0
D	6,238[1]	27	85	3.0	717.2	717.2	717.2	0.0
E	7,176[1]	30	131	1.7	720.6	720.6	720.7	0.1
F	7,952[1]	28	67	3.4	721.7	721.7	721.8	0.1
G	9,340[1]	36	141	1.6	727.8	727.8	727.9	0.1
7th Avenue Creek Tributary								
A	63[2]	21	44	5.6	719.3	719.3	719.4	0.1
B	718[2]	57	145	1.7	725.8	725.8	725.9	0.1
C	1,177[2]	15	82	3.0	730.5	730.5	730.6	0.1
Anderson Road Run	*	*	*	*	*	*	*	*
Anderson Road Run North Branch	*	*	*	*	*	*	*	*
Aurora Chain of Lakes								
A	961[3]	2,067	9,615	0.1	665.8	665.8	665.9	0.1
B	2,035[3]	761	5,382	0.2	665.8	665.8	665.9	0.1
C	2,890[3]	419	1,363	0.6	665.8	665.8	665.9	0.1
D	3,853[3]	375	3,282	2.8	665.9	665.9	666.0	0.1

[1]Feet above confluence with Fox River
[2]Feet above confluence with 7th Avenue Creek
[3]Feet above confluence with Blackberry Creek
[4]Elevation computed without consideration of backwater effects from Fox River
*Data not available

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

7TH AVENUE CREEK - 7TH AVENUE CREEK TRIBUTARY - ANDERSON ROAD RUN - ANDERSON ROAD RUN NORTH BRANCH - AURORA CHAIN OF LAKES

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Aurora Chain of Lakes (Continued)								
E	4,421[1]	432	1,431	0.6	667.2	667.2	667.2	0.0
F	5,141[1]	382	2,916	0.1	667.2	667.2	667.2	0.0
G	5,943[1]	293	1,629	0.3	667.3	667.3	667.3	0.0
H	6,576[1]	128	993	0.4	668.7	668.7	668.6	0.0
I	7,271[1]	181	1,389	0.3	668.7	668.7	668.6	0.0
J	7,780[1]	179	172	3.6	671.3	671.3	671.3	0.0
K	9,662[1]	308	2,420	0.2	672.6	672.6	672.6	0.0
L	10,436[1]	77	434	1.2	672.6	672.6	672.6	0.0
M	11,824[1]	149	1,187	1.3	672.7	672.7	672.7	0.0
N	12,295[1]	508	909	1.4	676.0	676.0	676.0	0.0
O	12,750[1]	386	1,561	0.3	676.1	676.1	676.1	0.0
P	13,571[1]	284	677	0.7	676.1	676.1	676.1	0.0
Q	14,008[1]	226	645	0.6	676.1	676.1	676.1	0.0
R	14,704[1]	327	469	0.8	676.2	676.2	676.2	0.0
S	15,006[1]	102	107	3.6	677.5	677.5	677.5	0.0
Aurora Chain of Lakes Cherry Hills Diversion								
A	803[2]	211	1,503	0.3	667.2	667.2	667.2	0.0
B	1,345[2]	274	883	0.8	667.2	667.2	667.2	0.0
C	2,231[2]	211	369	1.2	667.3	667.3	667.3	0.0
D	2,506[2]	85	88	5.0	667.8	667.8	667.8	0.0
E	3,489[2]	262	292	1.7	668.6	668.6	668.6	0.0

[1]Feet above confluence with Blackberry Creek
[2]Feet above confluence with Aurora Chain of Lakes

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

AURORA CHAIN OF LAKES - AURORA CHAIN OF LAKES CHERRY HILLS DIVERSION

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Aurora Chain of Lakes Cherry Hills Diversion (Continued)								
F	4,035[1]	140	317	1.3	668.8	668.8	668.8	0.0
G	4,764[1]	420	452	0.9	670.2	670.2	670.2	0.0
Big Rock Creek								
A	49,940[2]	*	*	*	658.2	658.2	*	*
B	56,687[2]	*	*	*	665.3	665.3	*	*
C	63,184[2]	*	*	*	672.8	672.8	*	*
D	66,739[2]	*	*	*	678.3	678.3	*	*
Blackberry Creek								
A	63,819[2]	452	2,271	1.2	660.0	660.0	660.0	0.0
B	67,324[2]	429	1,624	1.8	661.8	661.8	661.8	0.0
C	71,081[2]	636	3,091	1.0	663.8	663.8	663.7	0.0
D	72,970[2]	641	2,777	1.1	665.0	665.0	664.9	0.0
E	73,417[2]	693	2,318	1.3	665.7	665.7	665.7	0.0
F	76.619[2]	477	2,915	2.1	667.0	667.0	667.0	0.0
G	78,929[2]	559	2,506	2.4	668.6	668.6	668.7	0.0
H	79,324[2]	211	1,091	3.3	669.3	669.3	669.4	0.1
I	80,011[2]	523	2,601	1.9	669.9	669.9	670.0	0.1
J	80,334[2]	895	5,838	1.3	670.7	670.7	670.8	0.1
K	82,134[2]	475	3,597	1.4	671.1	671.1	671.2	0.1

[1]Feet above confluence with Aurora Chain of Lakes
[2]Feet above confluence with Fox River
*Data not available

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

AURORA CHAIN OF LAKES CHERRY HILLS DIVERSION – BIG ROCK CREEK – BLACKBERRY CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Blackberry Creek (Continued)								
L	82,338[1]	417	2,992	1.7	671.2	671.2	671.3	0.1
M	82,784[1]	455	2,717	2.5	671.5	671.5	671.6	0.1
N	83,100[1]	431	1,678	3.5	671.7	671.7	671.7	0.1
O	85,290[1]	762	3,414	1.7	673.4	673.4	673.4	0.1
P	87,635[1]	482	3,363	1.9	674.8	674.8	674.9	0.1
Q	91,979[1]	527	3,483	1.8	676.0	676.0	676.1	0.1
R	92,487[1]	568	3,751	1.7	676.6	676.6	676.7	0.0
S	95,548[1]	969	3,443	1.7	677.3	677.3	677.4	0.0
T	96,310[1]	1912	6,742	1.0	677.8	677.8	677.9	0.1
U	104,948[1]	460	1,832	2.4	686.6	686.6	686.7	0.0
V	105,379[1]	482	2,757	1.6	688.3	688.3	688.3	0.0
W	108,915[1]	365	1,930	2.1	690.9	690.9	690.9	0.0
X	109,285[1]	412	2,292	1.5	691.5	691.5	691.5	0.0
Y	114,088[1]	467	2,331	2.0	694.6	694.6	694.6	0.0
Z	120,569[1]	683	3,884	1.4	702.0	702.0	702.0	0.0
AA	121,210[1]	615	2,724	1.6	702.7	702.7	702.7	0.0
AB	124,477[1]	621	1,952	2.6	706.0	706.0	706.1	0.0
AC	125,096[1]	1103	3,377	2.2	707.2	707.2	707.1	0.0
AD	127,367[1]	665	1,864	2.5	708.4	708.4	708.4	0.0
AE	127,697[1]	650	2,586	1.7	709.4	709.4	709.4	0.0
AF	131,215[1]	237	738	2.8	712.9	712.9	713.0	0.1

[1]Feet above confluence with Fox River

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

BLACKBERRY CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE [1]	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Blackberry Creek (Continued)								
AG	131,857	216	767	3.4	714.8	714.8	714.9	0.1
AH	136,407	340	792	2.7	721.1	721.1	721.1	0.0
AI	139,767	186	827	2.2	727.6	727.6	727.6	0.0
AJ	139,956	348	1,788	1.6	728.4	728.4	728.4	0.0
AK	145,220	208	849	2.5	735.4	735.4	735.4	0.0
AL	145,472	240	994	2.3	736.1	736.1	736.1	0.0
AM	150,442	624	538	2.8	741.1	741.1	741.1	0.0
AN	152,644	343	762	2.1	744.6	744.6	744.6	0.0
AO	153,016	268	623	2.4	745.4	745.4	745.5	0.1
AP	158,428	247	527	1.3	753.6	753.6	753.6	0.0
AQ	161,120	358	863	1.8	759.6	759.6	759.6	0.0
AR	161,437	324	720	2.5	760.2	760.2	760.2	0.0
AS	162,763	272	720	2.8	764.0	764.0	764.1	0.0
AT	164,379	162	434	2.4	768.1	768.1	768.1	0.0
AU	168,443	144	299	3.3	782.0	782.0	782.0	0.0
AV	169,912	239	444	2.9	787.4	787.4	787.4	0.0
AW	171,413	567	956	1.7	795.2	795.2	795.2	0.0
AX	174,780	329	93	8.1	804.8	804.8	804.9	0.1
AY	176,549	320	477	1.5	811.1	811.1	811.1	0.0
AZ	176,895	508	1,088	1.1	813.5	813.5	813.5	0.0
BA	181,788	305	319	2.3	829.3	829.3	829.3	0.0
BB	183,780	206	536	1.7	843.3	843.3	843.3	0.0
BC	184,361	491	2,324	0.5	847.6	847.6	847.6	0.0
BD	185,436	264	332	2.0	848.3	848.3	848.3	0.0

[1]Feet above confluence with Fox

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE [1]	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Bowes Creek								
A	200	644	959	0.6	792.6	792.4[2]	792.4	0.0
B	1,600	432	475	1.3	795.6	795.6	795.6	0.0
C	3,050	885	761	0.8	797.1	797.1	797.1	0.0
D	3,600	49	168	3.7	797.8	797.8	797.8	0.0
E	4,650	42	157	3.8	802.3	802.3	802.4	0.1
F	6,650	128	196	3.1	812.0	812.0	812.1	0.1
G	8,650	29	123	4.9	823.5	823.5	823.5	0.0
H	10,003	22	125	4.8	833.9	833.9	833.9	0.0
I	10,343	282	728	0.8	834.6	834.6	834.6	0.0
J	10,843	166	282	2.1	834.7	834.7	834.7	0.0
K	11,273	30	69	8.7	837.5	837.5	837.5	0.0
L	11,773	56	214	2.8	840.9	840.9	841.0	0.1
M	11,973	36	114	5.3	841.3	841.3	841.4	0.1
N	12,683	14	82	6.0	848.9	848.9	848.9	0.0
O	12,913	136	329	1.5	850.1	850.1	850.1	0.0
P	14,013	161	148	3.3	854.7	854.7	854.7	0.0
Q	14,513	283	458	1.1	857.3	857.3	857.3	0.0
R	15,413	191	143	3.4	860.6	860.6	860.6	0.0
S	16,363	245	295	1.7	864.5	864.5	864.5	0.0
T	17,569	30	76	6.8	869.3	869.3	869.3	0.0
U	18,112	125	272	1.9	871.7	871.7	871.7	0.0
V	19,042	38	79	6.6	874.8	874.8	874.8	0.0
W	21,542	252	323	1.6	886.8	886.8	886.8	0.0
X	23,942	248	156	3.3	896.3	896.3	896.4	0.1

[1]Feet above confluence with Stony Creek

[2]Elevation computed without consideration of backwater effects from Stony Creek

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

BOWES CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Bowes Creek (continued)								
Y	25,142[1]	114	233	2.2	902.0	902.0	902.0	0.0
Z	26,407[1]	218	772	0.7	909.0	909.0	909.0	0.0
AA	26,597[1]	139	356	1.4	909.0	909.0	909.0	0.0
AB	26,717[1]	225	502	0.8	909.1	909.1	909.1	0.0
AC	27,642[1]	97	193	2.1	909.9	909.9	909.9	0.0
AD	28,717[1]	153	148	2.7	913.2	913.2	913.2	0.0
AE	30,017[1]	844	871	0.5	914.3	914.3	914.3	0.0
AF	30,897[1]	10	52	4.9	917.5	917.5	917.5	0.0
Bowes Creek South								
A	35[2]	275	670	0.3	909.0	909.0[3]	909.0	0.0
B	1,735[2]	27	34	6.4	909.3	909.3	909.3	0.0
C	2,505[2]	10	55	4.0	913.3	913.3	913.4	0.1
Brewster Creek								
A	364[4]	30	103	3.2	696.9	690.5[5]	690.6	0.1
B	1,631[4]	141	347	0.9	696.9	694.8[5]	694.9	0.1
C	3,205[4]	30	81	3.4	700.5	700.5	700.6	0.1
D	3,860[4]	22	62	4.4	707.1	707.1	707.1	0.0

[1]Feet above confluence with Stony Creek
[2]Feet above confluence with Bowes Creek
[3]Elevation computed without consideration of backwater effects from Bowes Creek
[4]Feet above mouth at Fox River
[5]Elevation computed without consideration of backwater effects from Fox River

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

BOWES CREEK - BOWES CREEK SOUTH - BREWSTER CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Carpenter Creek								
A	334[1]	17	111	6.0	723.0	723.0	723.0	0.0
B	370[1]	11	108	6.2	725.1	725.1	725.2	0.1
C	450[1]	101	571	1.2	725.9	725.9	726.0	0.1
D	1,148[1]	98	231	2.9	729.8	729.8	729.9	0.1
E	1,872[1]	96	285	2.3	736.5	736.5	736.6	0.1
F	1,987[1]	190	899	0.7	738.6	738.6	738.7	0.1
Duffin Drain								
A	955[2]	*	*	*	681.0	681.0	*	*
B	4,261[2]	*	*	*	689.3	689.3	*	*
C	5,830[2]	*	*	*	693.3	693.3	*	*
D	6,271[2]	*	*	*	701.3	701.3	*	*
E	12,647[2]	*	*	*	702.3	702.3	*	*
Eakin Creek								
A	710[3]	529	1,375	1.2	869.9	869.9	869.9	0.0
B	1,250[3]	510	1,582	1.9	870.0	870.0	870.0	0.0
C	2,125[3]	*	*	*	870.3	870.3	*	*
D	3,425[3]	*	*	*	870.4	870.4	*	*

[1]Feet above confluence with Fox River
[2]Feet above confluence with Sugar Grove Branch
[3]Feet above confluence with South Branch Kishwaukee River
*Data not available

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

CARPENTER CREEK – DUFFIN DRAIN – EAKIN CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
East Branch Big Rock Creek								
A	2,697[1]	*	*	*	694.1	694.1	*	*
B	6,760[1]	*	*	*	699.7	699.7	*	*
C	8,401[1]	*	*	*	702.4	702.4	*	*
D	10,206[1]	*	*	*	708.7	708.7	*	*
AA	60,294[1]	*	*	*	786.7	786.7	*	*
AB	65,228[1]	*	*	*	795.0	795.0	*	*
AC	71,180[1]	*	*	*	807.5	807.5	*	*
East Branch Big Rock Creek Tributary 2								
A	2,484[2]	*	*	*	820.4	820.4	*	*
B	4,662[2]	*	*	*	833.8	833.8	*	*
C	7,235[2]	*	*	*	842.8	842.8	*	*

[1]Feet above confluence with Big Rock Creek
[2]Feet above confluence with East Branch Big Rock Creek
*Data not available

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

EAST BRANCH BIG ROCK CREEK – EAST BRANCH BIG ROCK CREEK TRIBUTARY 2

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
East Run								
A	465[1]	701	2,886	0.7	674.1	674.1	674.1	0.0
B	2,768[1]	316	3,053	0.3	674.1	674.1	674.1	0.0
C	5,158[1]	446	4,204	0.2	674.2	674.2	674.2	0.0
D	5,659[1]	399	1,890	0.6	674.4	674.4	674.4	0.0
E	8,552[1]	449	1,397	0.8	675.3	675.3	675.2	0.0
F	9,352[1]	515	1,194	1.2	675.9	675.9	675.9	0.0
G	9,702[1]	371	1,021	1.4	676.0	676.0	676.0	0.0
H	10,685[1]	683	1,461	1.0	677.4	677.4	677.5	0.0
I	11,224[1]	404	2,117	0.3	681.7	681.7	681.7	0.0
J	11,439[1]	547	4,830	0.1	681.7	681.7	681.7	0.0
K	11,739[1]	599	3,922	0.1	681.7	681.7	681.7	0.0
L	12,119[1]	290	1,790	0.3	681.7	681.7	681.7	0.0
M	15,214[1]	341	694	2.2	683.0	683.0	683.0	0.0
N	15,539[1]	376	998	0.7	683.6	683.6	683.6	0.0
O	19,300[1]	58	160	2.9	690.6	690.6	690.6	0.0
P	20,582[1]	140	257	2.8	696.8	696.8	696.8	0.1
Q	20,897[1]	174	634	1.5	701.1	701.1	701.1	0.0
East Run North Branch								
A	2,679[2]	459	1,250	0.1	682.5	682.5	682.6	0.1
B	3,346[2]	400	936	0.1	682.5	682.5	682.6	0.1
C	3,471[2]	451	1,035	0.1	684.7	684.7	684.7	0.0

[1]Feet above confluence with Blackberry Creek
[2]Feet above confluence with East Run

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

EAST RUN - EAST RUN NORTH BRANCH

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
East Run North Loop								
A	380[1]	210	520	1.3	677.4	677.4	677.5	0.0
B	765[1]	327	1,303	0.5	680.3	680.3	680.3	0.1
C	1,553[1]	70	202	2.3	680.4	680.4	680.4	0.1
D	1,941[1]	375	2,499	0.1	682.4	682.4	682.5	0.1
Elburn Run								
A	1,133[2]	287	489	5.0	743.6	743.6	743.7	0.1
B	5,000[2]	101	124	8.4	768.7	768.7	768.8	0.1
C	5,307[2]	80	225	5.3	772.2	772.2	772.2	0.0
D	7,934[2]	85	225	4.0	790.1	790.1	790.1	0.0
E	8,464[2]	130	464	1.7	793.4	793.4	793.4	0.0
F	12,483[2]	234	259	1.9	806.0	806.0	806.0	0.0
G	12,761[2]	496	1,098	0.3	808.8	808.8	808.8	0.0
H	16,147[2]	217	98	1.3	824.6	824.6	824.6	0.0
I	16,362[2]	390	298	0.4	834.4	834.4	834.4	0.0
Ferson Creek								
A	1,357[3]	182	1,060	3.8	692.9	692.9	693.0	0.1
B	7,762[3]	274	1,369	2.9	706.1	706.1	706.2	0.1
C	11,352[3]	250	1,284	3.0	714.0	714.0	714.1	0.1
D	16,062[3]	318	1,115	3.5	727.4	727.4	727.4	0.0
E	22,840[3]	334	1,946	2.0	746.0	746.0	746.1	0.1
F	30,641[3]	951	4,115	0.6	753.0	753.0	753.0	0.0
G	30,881[3]	617	2,799	0.9	753.1	753.1	753.2	0.1

[1]Feet above confluence with East Run
[2]Feet above confluence with Blackberry Creek
[3]Feet above confluence with Fox River

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

EAST RUN NORTH LOOP - ELBURN RUN - FERSON CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Ferson Creek (Continued)								
H	31,441[1]	641	2,018	1.3	753.2	753.2	753.3	0.1
I	31,716[1]	766	2,272	1.1	753.3	753.3	753.4	0.1
J	31,766[1]	671	1,638	1.5	753.3	753.3	753.4	0.1
K	31,866[1]	748	1,903	1.3	753.5	753.5	753.6	0.1
L	32,076[1]	776	3,532	0.7	753.6	753.6	753.7	0.1
M	32,926[1]	982	2,721	0.9	753.7	753.7	753.8	0.1
N	34,461[1]	1,561	2,466	1.0	754.5	754.5	754.6	0.1
O	35,346[1]	715	1,123	0.8	755.6	755.6	755.7	0.1
P	35,656[1]	474	886	1.0	755.6	755.6	755.7	0.1
Q	36,543[1]	31	141	6.0	758.4	758.4	758.4	0.0
R	36,843[1]	267	436	1.9	760.3	760.3	760.3	0.0
S	37,988[1]	397	704	1.2	763.0	763.0	763.1	0.1
T	38,218[1]	228	410	2.1	763.5	763.5	763.6	0.1
U	38,830[1]	189	632	1.3	769.5	769.5	769.5	0.0
V	39,040[1]	114	207	4.0	769.5	769.5	769.5	0.0
W	39,330[1]	31	170	4.9	771.2	771.2	771.2	0.0
X	40,630[1]	259	849	1.0	772.9	772.9	773.0	0.1
Y	41,960[1]	957	2,453	0.2	773.1	773.1	773.2	0.1
Z	42,540[1]	1,359	3,892	0.1	773.1	773.1	773.2	0.1
AA	43,005[1]	1,751	2,647	0.2	773.1	773.1	773.2	0.1
AB	43,681[1]	320	273	1.9	773.1	773.1	773.2	0.1
AC	44,205[1]	42	138	3.7	774.3	774.3	774.3	0.0
AD	44,280[1]	201	305	1.7	775.6	775.6	775.6	0.0
AE	45,281[1]	24	108	5.2	778.5	778.5	778.5	0.0
AF	45,715[1]	163	420	1.3	779.6	779.6	779.6	0.0

[1]Feet above confluence with Fox River

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

FERSON CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Ferson Creek (Continued)								
AG	46,165[1]	36	124	4.5	780.0	780.0	780.1	0.1
AH	46,515[1]	87	162	3.5	781.4	781.4	781.4	0.0
AI	47,315[1]	1,375	2,388	0.2	781.9	781.9	781.9	0.0
AJ	47,815[1]	1,089	290	1.9	782.0	782.0	782.0	0.0
AK	48,665[1]	646	447	1.5	784.2	784.2	784.2	0.0
AL	49,075[1]	467	458	1.5	785.8	785.8	785.8	0.0
AM	49,575[1]	472	558	1.2	786.9	786.9	786.9	0.0
AN	50,335[1]	316	520	1.3	789.2	789.2	789.2	0.0
AO	51,905[1]	84	213	3.2	793.8	793.8	793.8	0.0
AP	53,015[1]	509	688	1.0	796.5	796.5	796.6	0.1
AQ	53,565[1]	158	308	2.2	797.5	797.5	797.6	0.1
AR	54,369[1]	107	501	1.4	804.2	804.2	804.2	0.0
AS	54,999[1]	278	1,126	0.6	804.3	804.3	804.3	0.0
AT	55,474[1]	44	241	2.8	805.3	805.3	805.3	0.0
AU	56,809[1]	353	1,686	0.3	814.1	814.1	814.2	0.1
AV	58,209[1]	537	3,227	0.2	814.2	814.2	814.3	0.1
AW	59,809[1]	425	878	0.6	814.2	814.2	814.3	0.1
AX	61,152[1]	180	160	3.2	816.7	816.7	816.7	0.0
AY	61,409[1]	159	203	2.2	820.0	820.0	820.1	0.1
AZ	62,809[1]	128	258	1.7	824.9	824.9	824.9	0.0
BA	65,109[1]	74	210	2.1	835.8	835.8	835.8	0.0
BB	65,930[1]	16	71	6.2	841.1	841.1	841.1	0.0
BC	66,145[1]	50	105	3.7	843.5	843.5	843.6	0.1
BD	68,275[1]	150	272	1.4	849.1	849.1	849.1	0.0
BE	69,675[1]	50	94	4.1	852.6	852.6	852.7	0.1

[1]Feet above confluence with Fox River

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

FERSON CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Ferson Creek (Continued)								
BF	71,015[1]	123	190	2.0	858.3	858.3	858.3	0.0
BG	72,149[1]	275	428	0.7	863.6	863.6	863.6	0.0
BH	72,839[1]	120	138	2.1	863.9	863.9	863.9	0.0
BI	73,439[1]	47	83	3.5	866.1	866.1	866.1	0.0
BJ	74,062[1]	68	114	2.6	868.4	868.4	868.4	0.0
BK	74,350[1]	54	61	4.8	869.6	869.6	869.6	0.0
BL	74,701[1]	19	56	5.2	872.0	872.0	872.0	0.0
Fitchie Creek								
A	1,880[2]	147	233	1.9	781.5	781.5	781.5	0.0
B	2,952[2]	261	366	1.2	783.6	783.6	783.6	0.0
C	3,732[2]	122	228	1.9	784.7	784.7	784.7	0.0
D	4,845[2]	20	97	3.5	788.2	788.2	788.3	0.1
E	5,230[2]	136	205	1.6	790.2	790.2	790.3	0.1
F	6,280[2]	174	168	2.0	794.5	794.5	794.5	0.0
G	7,090[2]	346	266	1.3	798.0	798.0	798.0	0.0
H	9,140[2]	78	124	2.7	805.8	805.8	805.8	0.0
I	9,755[2]	15	78	3.8	813.2	813.2	813.2	0.0
J	9,955[2]	235	347	0.9	813.7	813.7	813.7	0.0
K	10,305[2]	107	244	1.2	813.9	813.9	813.9	0.0
L	10,405[2]	42	48	6.2	813.9	813.9	813.9	0.0
M	11,716[2]	16	101	2.9	825.2	825.2	825.2	0.0

[1]Feet above confluence with Fox River
[2]Feet above confluence with Otter Creek

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

FERSON CREEK - FITCHIE CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Fitchie Creek (Continued)								
N	12,061[1]	242	445	0.7	825.6	825.6	825.6	0.0
O	12,261[1]	137	206	1.4	825.8	825.8	825.8	0.0
P	12,931[1]	100	147	2.0	829.1	829.1	829.1	0.0
Q	13,761[1]	178	291	1.0	832.1	832.1	832.1	0.0
R	14,651[1]	39	77	3.9	836.2	836.2	836.2	0.0
S	15,391[1]	103	182	1.6	839.3	839.3	839.3	0.0
T	16,447[1]	168	112	2.7	842.7	842.7	842.7	0.0
U	16,988[1]	126	118	2.5	843.9	843.9	844.0	0.1
V	18,188[1]	127	204	1.5	847.9	847.9	847.9	0.0
W	19,988[1]	69	99	3.2	854.5	854.5	854.5	0.0
X	21,588[1]	141	232	1.3	861.1	861.1	861.1	0.0
Y	22,668[1]	145	196	1.6	863.3	863.3	863.3	0.0
Z	23,143[1]	35	100	3.1	865.4	865.4	865.5	0.1
AA	23,693[1]	50	130	2.4	867.7	867.7	867.8	0.1
AB	25,513[1]	125	125	2.5	874.4	874.4	874.4	0.0
AC	26,163[1]	10	55	4.0	877.1	877.1	877.1	0.0
AD	26,313[1]	19	41	5.4	877.4	877.4	877.4	0.0
Four Winds Way Creek								
A	1,172[2]	59	133	2.7	729.4	729.4	729.5	0.1
B	1,928[2]	70	206	1.7	737.2	737.2	737.3	0.1
C	2,603[2]	29	97	3.7	747.3	747.3	747.4	0.1
D	3,630[2]	44	188	1.9	768.9	768.9	768.9	0.0

[1]Feet above confluence with Fox River

[2]Feet above confluence with Otter Creek

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

FITCHIE CREEK - FOUR WINDS WAY CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE [1]	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Fox River								
A	242,940	402	3,309	5.7	616.3	616.3	616.4	0.1
B	243,640	453	4,058	4.6	617.2	617.2	617.3	0.1
C	244,500	289	2,151	8.7	617.4	617.4	617.5	0.1
D	244,625	390	3,626	5.2	618.7	618.7	618.8	0.1
E	245,075	471	3,799	4.9	619.1	619.1	619.2	0.1
F	245,715	399	4,477	4.2	619.9	619.9	620.0	0.1
G	246,252	406	4,829	3.9	620.3	620.3	620.4	0.1
H	246,475	332	4,334	4.3	620.4	620.4	620.5	0.1
I	246,690	480	5,670	3.3	621.3	621.3	621.4	0.1
J	246,844	593	5,673	3.3	621.4	621.4	621.5	0.1
K	248,334	369	4,201	4.5	621.8	621.8	621.9	0.1
L	248,790	352	4,054	4.6	621.9	621.9	622.0	0.1
M	249,330	522	4,922	3.8	622.0	622.0	622.0	0.0
N	250,030	363	3,488	5.3	622.0	622.0	622.1	0.1
O	250,850	506	3,773	4.9	622.5	622.5	622.5	0.0
P	251,310	220	1,956	9.5	622.5	622.5	622.6	0.1
Q	251,550	430	3,471	5.4	623.3	623.3	623.4	0.1
R	252,290	609	5,261	3.5	624.1	624.1	624.2	0.1
S	252,640	648	6,069	3.1	624.2	624.2	624.3	0.1
T	252,930	774	5,442	3.4	624.3	624.3	624.4	0.1
U	253,260	190	1,687	5.9	625.7	625.7	625.8	0.1
V	254,210	411	2,938	3.5	626.9	626.9	626.9	0.0
W	254,570	497	2,802	3.6	627.1	627.1	627.1	0.0
X	254,870	480	4,904	3.7	627.3	627.3	627.4	0.1
Y	255,160	356	4,435	4.2	628.1	628.1	628.2	0.1
Z	255,390	360	4,557	4.1	628.2	628.2	628.3	0.1

[1]Feet above mouth at Illinois River

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

FOX RIVER

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE [1]	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Fox River (continued)								
AA	255,620	290	3,699	5.0	628.3	628.3	628.4	0.1
AB	256,100	340	3,610	5.2	628.5	628.5	628.6	0.1
AC	256,270	358	3,914	4.8	628.6	628.6	628.7	0.1
AD	256,520	463	5,570	3.3	629.0	629.0	629.1	0.1
AE	256,750	164	1,749	5.2	629.0	629.0	629.1	0.1
AF	257,250	161	1,633	5.6	629.2	629.2	629.3	0.1
AG	257,780	160	1,710	5.4	629.6	629.6	629.7	0.1
AH	257,970	164	1,596	5.8	629.7	629.7	629.8	0.1
AI	258,200	185	2,550	3.6	634.8	634.8	634.8	0.0
AJ	258,630	605	6,315	2.9	635.0	635.0	635.0	0.0
AK	259,100	648	6,484	2.9	635.0	635.0	635.0	0.0
AL	260,100	720	6,801	2.7	635.2	635.2	635.2	0.0
AM	262,310	840	7,253	2.6	635.6	635.6	635.7	0.1
AN	262,780	730	6,105	2.8	635.7	635.7	635.7	0.0
AO	263,140	710	6,953	2.5	635.8	635.8	635.8	0.0
AP	270,135	637	4,485	3.2	640.1	640.1	640.2	0.1
AQ	273,288	526	3,958	3.6	642.0	642.0	642.1	0.1
AR	274,972	594	3,425	4.2	643.2	643.2	643.3	0.1
AS	276,503	558	2,977	4.8	644.4	644.4	644.5	0.1
AT	277,380	652	5,674	2.5	645.0	645.0	645.1	0.1
AU	277,680	607	6,538	2.2	650.9	650.9	651.0	0.1
AV	279,254	754	6,158	2.3	651.0	651.0	651.1	0.1
AW	279,888	706	5,609	2.6	651.1	651.1	651.2	0.1
AX	282,781	776	4,440	3.2	651.6	651.6	651.7	0.1
AY	285,895	610	3,424	3.9	653.7	653.7	653.8	0.1
AZ	288,858	412	2,373	5.7	655.3	655.3	655.3	0.0

[1]Feet above mouth at Illinois River

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

FOX RIVER

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE [1]	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Fox River (continued)								
BA	290,100	675	4,720	2.9	658.2	658.2	658.3	0.1
BB	292,270	435	3,974	3.4	659.7	659.7	659.8	0.1
BC	294,050	475	4,293	3.1	663.0	663.0	663.1	0.1
BD	297,040	475	5,125	2.6	668.0	668.0	668.1	0.1
BE	300,790	534	4,871	2.8	669.2	669.2	669.3	0.1
BF	301,942	522	4,343	3.1	669.5	669.5	669.6	0.1
BG	304,999	459	3,791	3.6	672.3	672.3	672.4	0.1
BH	307,439	425	3,431	3.9	673.8	673.8	673.9	0.1
BI	308,772	440	1,694	7.2	678.9	678.9	679.0	0.1
BJ	313,316	577	4,830	2.5	681.5	681.5	681.6	0.1
BK	319,303	360	3,417	3.6	686.2	686.2	686.3	0.1
BL	320,417	338	4,270	2.9	688.7	688.7	688.9	0.1
BM	325,016	843	5,783	2.1	691.7	691.7	691.8	0.1
BN	330,818	705	8,010	1.5	692.6	692.6	692.7	0.1
BO	333,000	885	7,761	1.6	692.9	692.9	693.0	0.1
BP	339,200	592	5,964	2.1	693.8	693.8	693.9	0.1
BQ	341,832	776	6,765	1.8	694.5	694.5	694.6	0.1
BR	344,008	738	7,017	1.7	695.0	695.0	695.1	0.1
BS	347,604	637	6,629	1.8	695.6	695.6	695.7	0.1
BT	351,204	726	6,368	1.9	696.4	696.4	696.5	0.1
BU	353,781	510	4,450	2.8	697.3	697.3	697.4	0.1
BV	355,117	893	6,772	1.8	697.7	697.7	697.8	0.1
BW	356,400	819	4,436	2.8	698.0	698.0	698.1	0.1
BX	357,625	726	5,050	2.4	698.9	698.9	699.0	0.1
BY	359,425	438	4,378	2.8	699.7	699.7	699.8	0.1

[1]Feet above mouth at Illinois River

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

FOX RIVER

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE [1]	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Fox River (continued)								
BZ	359,874	512[2]	3,492	3.5	700.3	700.3	700.4	0.1
CA	359,964	367	1,301	8.7	702.9	702.9	703.0	0.1
CB	363,232	795	6,150	1.8	705.0	705.0	705.0	0.0
CC	365,355	617	5,802	1.9	705.4	705.4	705.4	0.0
CD	367,995	620	4,211	2.7	706.2	706.2	706.3	0.1
CE	370,862	390	3,204	3.5	708.1	708.1	708.2	0.1
CF	373,037	522	5,572	2.0	709.2	709.2	709.3	0.1
CG	374,658	312	3,728	3.0	709.6	709.6	709.7	0.1
CH	376,015	384	4,450	2.5	709.9	709.9	710.0	0.1
CI	377,552	276	4,074	2.8	710.2	710.2	710.3	0.1
CJ	379,326	342	4,183	2.7	711.1	711.1	711.2	0.1
CK	379,621	456	5,782	1.8	713.3	713.3	713.3	0.0
CL	381,396	720	6,117	1.7	713.5	713.5	713.5	0.0
CM	384,653	492	2,928	3.6	714.1	714.1	714.1	0.0
CN	387,061	517	5,643	1.9	714.9	714.9	715.0	0.1
CO	389,516	652	6,004	1.8	715.2	715.2	715.2	0.0
CP	391,116	644	7,038	1.5	715.4	715.4	715.4	0.0
CQ	392,769	576	5,783	1.8	715.5	715.5	715.6	0.1
CR	395,700	619	5,758	1.8	715.8	715.8	715.9	0.1
CS	397,400	1,101	7,398	1.4	716.1	716.1	716.2	0.1
CT	402,518	469	2,669	3.9	717.8	717.8	717.9	0.1
CU	404,511	281	2,865	3.7	719.6	719.6	719.7	0.1
CV	406,439	381	3,508	3.0	720.0	720.0	720.1	0.1
CW	407,468	374	3,841	2.7	720.4	720.4	720.5	0.1
CX	409,944	227	2,352	4.5	721.2	721.2	721.3	0.1

[1]Feet above mouth at Illinois River

[2]Floodway width reflects constricted section, see FIRM panel for regulatory floodway

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL
AND INCORPORATED AREAS

FLOODWAY DATA

FOX RIVER

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Fox River (Continued)								
CY	410,874[1]	708	5,994	1.8	722.1	722.1	722.2	0.1
CZ	414,015[1]	575	4,181	2.4	725.1	725.1	725.2	0.1
DA	417,841[1]	558	4,139	2.4	726.9	726.9	727.0	0.1
DB	420,346[1]	650	4,101	2.5	728.2	728.2	728.3	0.1
DC	422,521[1]	643	4,035	2.5	729.0	729.0	729.1	0.1
DD	424,528[1]	787	5,117	2.0	729.9	729.9	730.0	0.1
DE	428,504[1]	878	5,043	2.0	731.8	731.8	731.9	0.1
Fox River East Channel								
A	253,290[2]	144	1,335	6.3	624.8	624.8	624.8	0.0
B	254,170[2]	156	1,471	5.7	626.1	626.1	626.2	0.1
C	254,360[2]	160	1,261	6.7	626.3	626.3	626.4	0.1
D	257,060[2]	164	1,815	5.2	629.1	629.1	629.2	0.1
E	257,530[2]	162	1,725	5.5	629.3	629.3	629.4	0.1
F	258,020[2]	171	1,580	6.0	629.4	629.4	629.5	0.1
G	258,240[2]	171	1,680	5.9	629.6	629.6	629.7	0.1
H	258,470[2]	235	2,716	3.5	634.8	634.8	634.8	0.0
Fox River Tributary								
A	2,300[3]	250	223	1.61	640.6	640.6	640.7	0.1

[1]Feet above mouth at Illinois River
[2]Feet above confluence with Fox River
[3]Feet above confluence with Fox River Tributary (East Branch)

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

FOX RIVER - FOX RIVER EAST CHANNEL - FOX RIVER TRIBUTARY

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Fox River Tributary East Branch								
A	400[1]	166	197	0.4	635.6	635.6	635.7	0.1
B	1,750[1]	126	190	0.4	635.7	635.7	635.8	0.1
C	1,800[1]	116	162	0.5	635.7	635.7	635.8	0.1
D	2,720[1]	94	97	0.8	636.3	636.3	636.4	0.1
Geneva Creek								
A	877[2]	56	172	3.7	682.2	682.2	682.3	0.1
B	2,365[2]	10	76	7.1	698.8	698.8	698.9	0.1
C	3,103[2]	100	321	2.7	705.2	705.2	705.3	0.1
Hampshire Creek								
A	15,840[3]	121	508	2.8	868.6	868.6	868.6	0.0
B	16,443[3]	216	535	2.6	870.9	870.9	870.9	0.0
C	16,854[3]	311	443	3.2	872.9	872.9	873.0	0.1
D	16,969[3]	471	579	2.2	873.4	873.4	873.4	0.0
E	17,447[3]	444	616	2.0	874.8	874.8	874.8	0.0
F	18,149[3]	787	574	2.0	877.1	877.1	877.2	0.1
G	18,561[3]	725	629	1.8	878.4	878.4	878.4	0.0
H	19,400[3]	222	427	2.7	880.8	880.8	880.8	0.0
I	19,867[3]	281	385	3.0	882.1	882.1	882.1	0.0
J	20,003[3]	426	562	2.1	882.6	882.6	882.6	0.0
K	20,252[3]	906	2,789	0.4	884.9	884.9	884.9	0.0

[1]Feet above confluence with Fox River Tributary
[2]Feet above confluence with Fox River
[3]Feet above confluence with Burlington Creek

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

FOX RIVER TRIBUTARY EAST BRANCH - GENEVA CREEK - HAMPSHIRE CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE [1]	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Hampshire Creek (continued)								
L	20,651	995	2,308	0.4	884.9	884.9	884.9	0.0
M	21,362	286	959	1.0	885.4	885.4	885.4	0.0
N	21,660	59	176	5.6	888.2	888.2	888.3	0.1
O	22,128	163	346	2.9	891.4	891.4	891.5	0.1
P	22,605	326	479	2.1	893.1	893.1	893.1	0.0
Q	22,868	406	872	1.1	893.9	893.9	893.9	0.0
R	23,806	178	393	2.5	896.5	896.5	896.6	0.1
S	24,200	194	416	2.4	897.6	897.6	897.6	0.0
T	24,673	243	262	3.1	899.3	899.3	899.3	0.0
U	25,296	153	325	2.5	903.7	903.7	903.8	0.1
V	25,728	156	388	2.1	905.7	905.7	905.8	0.1
W	25,947	122	274	2.0	906.4	906.4	906.4	0.0
X	26,424	111	171	3.3	909.0	909.0	909.0	0.0
Y	26,845	116	168	3.3	911.7	911.7	911.8	0.1
Z	27,477	227	305	1.5	917.2	917.2	917.3	0.1
AA	27,883	313	373	1.3	920.1	920.1	920.1	0.0
AB	28,548	450	449	1.0	923.3	923.3	923.3	0.0
AC	29,298	343	485	1.0	925.4	925.4	925.4	0.0
AD	29,808	348	373	1.1	928.4	928.4	928.4	0.0
AE	30,394	351	351	1.2	934.5	934.5	934.6	0.1
AF	30,795	135	215	1.9	938.7	938.7	938.7	0.0
AG	31,046	130	194	2.1	940.8	940.8	940.8	0.0
AH	31,404	146	271	1.5	943.0	943.0	943.0	0.0
AI	31,685	205	279	1.5	944.9	944.9	944.9	0.0
AJ	31,897	94	142	1.8	946.5	946.5	946.5	0.0

[1]Feet above confluence with Burlington Creek

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

HAMPSHIRE CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Hampshire Creek (continued)								
AK	32,421[1]	189	148	1.7	950.3	950.3	950.3	0.0
AL	32,593[1]	176	152	1.6	951.4	951.4	951.4	0.0
AM	32,842[1]	267	213	1.1	953.3	953.3	953.3	0.0
AN	33,360[1]	316	245	1.0	958.4	958.4	958.4	0.0
AO	33,624[1]	99	81	1.7	960.3	960.3	960.3	0.0
Hampshire Creek South								
A	0.064[2]	16	74	3.7	873.1	872.6[3]	872.6	0.0
B	0.138[2]	44	110	2.5	875.8	875.8	875.8	0.0
C	0.319[2]	94	299	1.7	877.8	877.8	877.9	0.1
D	0.478[2]	75	255	1.9	880.7	880.7	880.7	0.0
E	0.612[2]	115	302	1.5	881.0	881.0	881.0	0.0
F	0.915[2]	46	170	2.7	890.5	890.5	890.6	0.1
G	1.135[2]	25	88	4.7	894.2	894.2	894.3	0.1
H	1.232[2]	60	250	1.7	897.6	897.6	897.7	0.1
I	1.310[2]	51	240	1.7	899.0	899.0	899.1	0.1
J	1.605[2]	48	133	2.2	903.7	903.7	903.8	0.1
Hampshire Creek Tributary No. 1	*	*	*	*	*	*	*	*

[1]Feet above confluence with Burlington Creek
[2]Miles above confluence with Hampshire Creek
[3]Elevations computed without consideration of backwater effects from Hampshire Creek
*Data not available

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

HAMPSHIRE CREEK - HAMPSHIRE CREEK SOUTH - HAMPSHIRE CREEK TRIBUTARY NO. 1

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Hampshire Creek Tributary No. 2	*	*	*	*	*	*	*	*
Hampshire Creek Tributary No. 3	*	*	*	*	*	*	*	*
Hampshire Creek Tributary No. 4	*	*	*	*	*	*	*	*
Indian Creek								
A	65[1]	105	249	8.7	635.4	631.5	631.5	0.0
B	90[1]	185	361	6.1	635.4	632.4	632.5	0.1
C	330[1]	127	259	10.1	637.6	637.6	637.6	0.0
D	425[1]	133	435	7.5	639.7	639.7	639.7	0.0
E	620[1]	261	1,253	2.7	641.5	641.5	641.5	0.0
F	715[1]	110	398	5.6	643.8	643.8	643.8	0.0
G	1,000[1]	83	247	13.7	645.1	645.1	645.2	0.1
H	1,050[1]	230	847	3.8	649.8	649.8	649.5	0.0
I	1,280[1]	62	512	6.0	649.9	649.9	650.0	0.1
J	1,680[1]	94	744	4.1	651.3	651.3	651.4	0.1
K	2,080[1]	40	280	10.9	653.5	653.5	653.4	0.0
L	2,180[1]	52	357	8.6	657.9	657.9	657.9	0.0
M	3,780[1]	95	419	7.3	668.1	668.1	668.2	0.1
N	4,655[1]	154	762	4.0	672.6	672.6	672.7	0.1
O	7,585[1]	49	365	5.0	685.9	685.9	686.0	0.1
P	7,720[1]	40	306	5.9	686.4	686.4	686.5	0.1

[1]Feet above confluence with Fox River

* Data not available

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

HAMPSHIRE CREEK TRIBUTARY NO. 2 - HAMPSHIRE CREEK TRIBUTARY NO. 3 - HAMPSHIRE CREEK TRIBUTARY NO. 4 - INDIAN CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Indian Creek (Continued)								
Q	9,130[1]	133	514	3.6	690.4	690.4	690.5	0.1
R	9,295[1]	73	454	4.1	691.5	691.5	691.6	0.1
S	9,485[1]	76	375	5.0	691.9	691.9	692.0	0.1
T	9,765[1]	272	1,397	1.3	695.6	695.6	695.7	0.1
U	10,365[1]	283	1,275	1.5	695.8	695.8	695.9	0.1
V	10,415[1]	317	1,247	1.5	695.8	695.8	695.9	0.1
W	11,745[1]	144	531	3.3	697.3	697.3	697.4	0.1
X	11,885[1]	104	473	3.7	699.1	699.1	699.2	0.1
Y	13,095[1]	178	329	4.2	700.6	700.6	700.6	0.1
Z	13,270[1]	141	750	1.9	702.6	702.6	702.6	0.0
AA	14,110[1]	70	269	4.2	702.9	702.9	702.9	0.0
AB	14,375[1]	43	260	4.3	704.6	704.6	704.7	0.1
AC	14,880[1]	209	633	1.8	705.5	705.5	705.6	0.1
AD	15,230[1]	55[2]	273	4.0	705.8	705.8	705.9	0.1
AE	15,355[1]	129	564	2.0	706.8	706.8	706.9	0.1
AF	16,185[1]	111	434	2.5	707.4	707.4	707.5	0.1
AG	16,450[1]	106	492	2.2	707.6	707.6	707.7	0.1
AH	16,606[1]	152	293	2.9	707.6	707.6[3]	707.7	0.1
AI	16,922[1]	45	212	4.0	708.3	708.3[3]	708.3	0.0
AJ	17,466[1]	42	229	3.6	709.1	709.1[3]	709.2	0.1
AK	17,709[1]	43	275	2.9	709.6	709.6[3]	709.6	0.0
AL	19,980[1]	265	285	3.0	712.8	712.8	712.8	0.0
AM	20,175[1]	314	313	2.7	713.4	713.4	713.4	0.0

[1]Feet above confluence with Fox River

[2]Floodway width reflects constricted section, see FIRM panel for regulatory floodway

[3]The mapped floodplain elevation and flood profile at these cross sections are based on the critical duration storm and is at a higher elevation

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

INDIAN CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Indian Creek (Continued)								
AN	20,803[1]	750	197	3.0	714.7	714.7	714.7	0.1
AO	22,055[1]	142	155	4.0	716.3	716.3	716.3	0.1
AP	27,878[1]	293	467	1.4	727.1	727.1	727.2	0.1
AQ	29,584[1]	228	319	2.0	730.6	730.6	730.6	0.1
AR	29,821[1]	192	570	1.1	731.8	731.8	731.8	0.1
AS	32,805[1]	135	114	3.9	733.4	733.4	733.5	0.1
Indian Creek Prairie Path Run	*	*	*	*	*	*	*	*
Jelkes Creek								
A	364[2]	499	1,153	1.0	716.2	713.3[3]	713.4	0.1
B	2,978[2]	429	744	1.6	720.3	720.3	720.3	0.0
C	6,278[2]	466	755	1.6	724.9	724.9	724.9	0.0
D	7,867[2]	119	745	1.4	732.9	732.9	733.0	0.1
E	12,244[2]	317	883	0.9	739.6	739.6	739.6	0.0
F	13,453[2]	601	1,102	0.7	740.2	740.2	740.3	0.1
G	16,099[2]	212	330	2.5	747.7	747.7	747.8	0.1
H	17,070[2]	50	203	3.5	751.6	751.6	751.6	0.0
I	17,327[2]	296	916	0.8	753.8	753.8	753.8	0.0
J	17,614[2]	167	604	1.2	755.5	755.5	755.5	0.0
K	20,249[2]	41	103	6.0	768.6	768.6	768.6	0.0

[1]Feet above confluence with Fox River
[2]Feet above mouth at Fox River
[3]Elevation computed without consideration of backwater effects from Fox River
*Data not available

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

INDIAN CREEK - INDIAN CREEK PRAIRIE PATH RUN - JELKES CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Jericho Lake Diversion								
A	0[1]	*	*	*	656.5	656.5	*	*
B	480[1]	*	*	*	661.0	661.0	*	*
C	5,090[1]	*	*	*	661.9	661.9	*	*
D	6,120[1]	*	*	*	663.3	663.3	*	*
Lake Run								
A	1,436[2]	455	2,242	0.6	677.4	677.4	677.5	0.1
B	2,555[2]	432	5,031	0.4	677.4	677.4	677.5	0.1
C	2,806[2]	305	4,003	0.5	678.1	678.1	678.2	0.1
D	9,862[2]	62	349	2.2	683.4	683.4	683.4	0.0
E	10,419[2]	155	2,221	0.7	683.8	683.8	683.8	0.0
F	10,981[2]	117	3,228	0.7	684.2	684.2	684.2	0.0
G	14,498[2]	800	1,739	1.2	686.7	686.7	686.7	0.0
H	18,290[2]	480	1,365	2.0	687.3	687.3	687.2	0.0
I	18,702[2]	547	972	3.4	688.7	688.7	688.7	0.0
J	24,879[2]	47	259	2.9	692.7	692.7	692.7	0.0

[1]Feet above downstream limit of detailed study
[2]Feet above confluence with Blackberry Creek
*Data not available

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

JERICHO LAKE DIVERSION - LAKE RUN

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Lake Run (Continued)								
K	25,132[1]	63	182	4.0	693.8	693.8	693.8	0.0
L	30,436[1]	207	368	3.7	705.6	705.6	705.6	0.0
M	33,206[1]	23	108	4.2	727.3	727.3	727.3	0.0
N	33,530[1]	72	616	1.0	732.7	732.7	732.7	0.0
O	33,811[1]	169	779	1.0	732.7	732.7	732.7	0.0
P	34,193[1]	168	1,608	0.6	734.3	734.3	734.3	0.0
Q	38,533[1]	218	233	2.0	748.3	748.3	748.3	0.0
R	39,548[1]	156	105	1.6	752.5	752.5	752.5	0.0
S	39,968[1]	230	180	1.1	757.8	757.8	757.8	0.0
T	41,945[1]	140	82	1.1	769.6	769.6	769.6	0.0
U	42,766[1]	75	61	1.5	779.8	779.8	779.8	0.0
Lake Run Main Street Branch								
A	488[2]	217	567	0.7	706.2	706.2	706.2	0.0
B	2,410[2]	47	113	2.1	707.9	707.9	707.9	0.0
C	3,340[2]	315	357	1.1	708.1	708.1	708.1	0.0
D	4,460[2]	154	802	0.2	708.1	708.1	708.1	0.0
E	6,065[2]	100	112	1.1	708.4	708.4	708.5	0.0

[1]Feet above confluence with Blackberry Creek
[2]Feet above confluence with Lake Run

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

LAKE RUN - LAKE RUN MAIN STREET BRANCH

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Lake Run								
Nelson Lake Branch								
A	200[1]	626	956	0.2	694.8	694.8	694.8	0.0
B	400[1]	480	915	0.2	694.8	694.8	694.8	0.0
C	1,220[1]	228	240	0.3	694.8	694.8	694.8	0.0
D	5,130[1]	1391	2,004	0.0	694.8	694.8	694.8	0.0
E	7,850[1]	776	894	0.1	694.8	694.8	694.8	0.0
Lake Run North of I-88 Overflow	*	*	*	*	*	*	*	*
Lake Run North of I-88 Overflow East Branch	*	*	*	*	*	*	*	*
Lake Run South of I-88 Diversion								
A	891[1]	330	1,175	0.5	679.6	679.6	679.6	0.0
B	5,451[1]	381	625	0.8	681.4	681.4	681.3	0.0
Lord's Park Tributary								
A[4]	500[2]	161	348	3.2	717.7	717.7[3]	717.8	0.1
B[4]	2,185[2]	22	109	4.1	723.7	721.1[3]	721.2	0.1
C[4]	3,375[2]	495	953	0.7	725	721.5[3]	721.6	0.1

[1]Feet above confluence with Lake Run
[2]Feet above mouth at Poplar Creek
[3]Elevation computed without consideration of backwater effects from Poplar Creek
[4]Portions of this floodway data are duplicated within the countywide FIS for Cook County, Illinois and Unincorporated Areas
*Data not available

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

LAKE RUN NELSON LAKE BRANCH - LAKE RUN NORTH OF I-88 OVERFLOW - LAKE RUN NORTH OF I-88 OVERFLOW EAST BRANCH - LAKE RUN SOUTH OF I-88 DIVERSION - LORD'S PARK TRIBUTARY

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Mahoney Creek								
A	1,378[1]	34	137	3.0	674.4	674.4	674.5	0.1
B	3,265[1]	201	393	1.0	691.0	691.0	691.1	0.1
C	3,560[1]	46	126	3.2	692.2	692.2	692.2	0.0
D	4,759[1]	52	163	2.5	699.1	699.1	699.1	0.0
E	5,862[1]	32	68	6.0	701.8	701.8	701.8	0.0
F	7,140[1]	56	163	1.7	707.1	707.1	707.1	0.0
G	8,354[1]	75	39	4.0	714.8	714.8	714.8	0.0
H	10,746[1]	28	40	3.4	729.5	729.5	729.5	0.0
I	11,733[1]	85	83	1.5	736.3	736.3	736.3	0.0
Malgren Drain								
A	3,181[2]	*	*	*	709.0	709.0	*	*
B	9,250[2]	*	*	*	731.3	731.3	*	*
McKee Road Tributary								
A	3,949[3]	66	139	4.4	701.4	701.4	701.4	0.0
B	6,563[3]	257	736	0.8	705.2	705.2	705.2	0.0
C	9,203[3]	396	1,446	0.4	705.9	705.9	705.9	0.0
D	10,592[3]	565	3,067	0.1	706.2	706.2	706.2	0.0
E	11,535[3]	218	439	1.2	708.0	708.0	708.0	0.0
F	14,078[3]	500	227	2.3	708.4	708.4	708.5	0.1
G	14,997[3]	560	455	1.2	710.1	710.1	710.1	0.0
H	15,360[3]	30	105	3.2	712.5	712.5	712.6	0.1
Mill Creek								
A	850[4]	41	402	9.0	657.8	657.8	657.8	0.0
B	1,917[4]	109	836	4.3	661.6	661.6	661.7	0.1
C	2,893[4]	75	417	8.6	663.3	663.3	663.4	0.1

[1]Feet above confluence with Fox River
[2]Feet above confluence with East Branch Big Rock Creek
[3]Feet above mouth at Mill Creek
[4]Feet above mouth
*Date not available

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

MAHONEY CREEK - MALGREN DRAIN - MCKEE ROAD TRIBUTARY - MILL CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE [1]	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Mill Creek (continued)								
D	3,939	130	921	3.9	671.0	671.0	671.1	0.1
E	13,042	343	1,199	2.8	691.7	691.7	691.7	0.0
F	14,768	251	1,458	2.3	695.9	695.9	695.9	0.0
G	15,703	513	2,442	1.4	697.4	697.4	697.5	0.1
H	16,706	672	3,279	1.0	698.4	698.4	698.5	0.1
I	19,816	566	3,208	1.0	698.9	698.9	699.0	0.1
J	20,951	249	1,638	1.6	700.7	700.7	700.8	0.1
K	23,454	1,443	3,860	0.7	701.6	701.6	701.7	0.1
L	26,009	326	753	3.4	703.7	703.7	703.8	0.1
M	28,875	93	468	3.6	709.5	709.5	709.5	0.0
N	35,185	263	786	2.2	719.2	719.2	719.3	0.1
O	38,970	60	256	6.6	726.2	726.2	726.2	0.0
P	43,525	265	897	1.9	738.7	738.7	738.8	0.1
Q	45,745	407	709	2.4	742.3	742.3	742.4	0.1
R	50,175	242	861	2.0	756.3	756.3	756.4	0.1
S	53,615	404	750	2.3	763.4	763.4	763.5	0.1
T	55,050	320	553	2.3	768.1	768.1	768.2	0.1
U	55,557	344	938	1.2	771.3	771.3	771.4	0.1
V	56,447	140	337	3.4	771.9	771.9	772.0	0.1
W	57,247	129	310	3.7	775.2	775.2	775.3	0.1
X	57,937	182	826	1.4	779.4	779.4	779.5	0.1
Y	58,457	186	466	2.5	657.8	779.7	779.8	0.1
Z	59,357	250	607	1.8	661.6	783.8	783.9	0.1
AA	60,310	400	1,218	0.9	784.8	784.8	784.9	0.1
AB	60,890	334	890	1.2	785.3	785.3	784.9	0.1
AC	61,600	495	1,250	0.9	785.9	785.9	786.0	0.1

[1]Feet above mouth

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

MILL CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Mill Creek (Continued)								
AD	62,430[1]	820	1,140	0.9	786.5	786.5	786.6	0.1
AE	63,460[1]	490	812	1.2	787.6	787.6	787.6	0.0
AF	63,980[1]	540	648	1.5	788.7	788.7	788.7	0.0
AG	64,550[1]	570	813	1.2	789.9	789.9	790.0	0.1
AH	65,700[1]	652[2]	809	1.2	792.6	792.6	792.6	0.0
AI	66,500[1]	289	381	2.2	796.2	796.2	796.2	0.0
AJ	66,630[1]	298	602	1.4	799.0	799.0	799.0	0.0
AK	67,200[1]	387	976	0.9	801.5	801.5	801.5	0.0
AL	67,500[1]	202	423	2.0	801.7	801.7	801.7	0.0
AM	69,090[1]	335	427	2.0	806.3	806.3	806.3	0.0
AN	70,620[1]	136	307	2.8	813.1	813.1	813.2	0.1
AO	71,090[1]	264	443	1.9	814.9	814.9	814.9	0.0
AP	71,260[1]	185	284	3.0	815.5	815.5	815.5	0.0
AQ	71,500[1]	248	451	1.9	816.5	816.5	816.5	0.0
AR	72,000[1]	494	2,056	0.3	823.1	823.1	823.1	0.0
AS	72,190[1]	626	2,556	0.3	823.1	823.1	823.1	0.0
Mill Creek Tributary No. 2								
A	1,440[1]	310	859	0.3	792.9	792.9	792.9	0.0
B	2,500[1]	690	2,243	0.1	792.9	792.9	792.9	0.0
North Arm Brewster Creek								
A	137[1]	20	45	2.9	696.9	694.3[3]	694.4	0.1

[1]Feet above mouth
[2]Combined Mill Creek/Mill Creek Tributary No. 2 floodway
[3]Elevation computed without consideration of backwater effects from Fox River

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

MILL CREEK - MILL CREEK TRIBUTARY NO. 2 - NORTH ARM BREWSTER CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Norton Creek								
A	1,774[1]	334	1,411	0.7	695.7	695.7	695.8	0.1
B	2,503[1]	124	602	1.6	698.5	698.5	698.6	0.1
C	6,621[1]	74	268	3.7	715.3	715.3	715.4	0.1
D	15,196[1]	172	562	1.5	736.4	736.4	736.5	0.1
E	17,134[1]	194	1,202	0.6	741.0	741.0	741.1	0.1
Norton Creek Tributary	*	*	*	*	*	*	*	*
Otter Creek								
A	140[2]	1,308	2,340	1.1	755.6	755.6	755.6	0.0
B	980[2]	1,868	2,436	1.0	756.7	756.7	756.8	0.1
C	1,100[2]	1,919	3,069	0.8	756.8	756.8	756.9	0.1
D	1,370[2]	1,976	3,624	0.7	756.9	756.9	757.0	0.1
E	1,740[2]	1,618	2,411	1.0	757.1	757.1	757.2	0.1
F	1,940[2]	1,767	4,338	0.6	757.2	757.2	757.3	0.1
G	2,300[2]	1,504	3,613	0.7	757.3	757.3	757.4	0.1
H	2,540[2]	1,450	2,516	1.0	757.5	757.5	757.6	0.1
I	3,175[2]	781	1,273	2.0	758.7	758.7	758.7	0.0
J	3,675[2]	631	1,920	1.3	759.9	759.9	760.0	0.1
K	4,205[2]	787	849	2.6	760.2	760.2	760.3	0.1
L	4,645[2]	445	1,209	1.9	762.5	762.5	762.5	0.0
M	5,080[2]	697	1,174	1.9	763.0	763.0	763.1	0.1
N	5,780[2]	686	1,554	1.4	763.8	763.8	763.9	0.1
O	6,050[2]	668	1,638	1.4	764.0	764.0	764.1	0.1

[1]Feet above mouth
[2]Feet above confluence with Ferson Creek
*Data not available

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

NORTON CREEK - NORTON CREEK TRIBUTARY - OTTER CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE [1]	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Otter Creek (continued)								
P	6,490	767	1,803	1.2	764.2	764.2	764.3	0.1
Q	6,895	815	2,120	1.1	764.6	764.6	764.7	0.1
R	7,105	394	1,026	2.2	764.7	764.7	764.8	0.1
S	7,305	543	1,115	2.0	765.1	765.1	765.2	0.1
T	7,505	491	1,251	1.8	765.4	765.4	765.5	0.1
U	7,905	573	1,503	1.5	765.8	765.8	765.9	0.1
V	8,115	464	1,333	1.7	766.1	766.1	766.2	0.1
W	8,315	447	1,386	1.6	766.3	766.3	766.4	0.1
X	8,515	522	1,686	1.3	766.4	766.4	766.5	0.1
Y	8,715	556	1,890	1.2	766.5	766.5	766.6	0.1
Z	8,935	576	1,699	1.3	766.6	766.6	766.7	0.1
AA	9,135	509	1,490	1.5	766.7	766.7	766.8	0.1
AB	9,335	457	1,159	1.9	766.9	766.9	767.0	0.1
AC	9,735	629	1,555	1.4	767.5	767.5	767.6	0.1
AD	10,336	1,024	2,504	0.9	768.3	768.3	768.4	0.1
AE	10,536	532	1,581	1.4	768.4	768.4	768.5	0.1
AF	10,936	452	1,445	1.5	768.9	768.9	769.0	0.1
AG	11,508	558	2,401	0.9	770.9	770.9	771.0	0.1
AH	11,708	478	1,954	1.1	771.0	771.0	771.1	0.1
AI	11,908	498	1,784	1.2	771.1	771.1	771.2	0.1
AJ	12,108	495	2,025	1.1	771.2	771.2	771.3	0.1
AK	12,308	650	2,834	0.8	771.2	771.2	771.3	0.1
AL	12,508	607	1,905	1.1	771.3	771.3	771.4	0.1
AM	12,708	563	1,642	1.3	771.4	771.4	771.5	0.1
AN	12,908	510	1,644	1.3	771.5	771.5	771.6	0.1
AO	13,108	578	1,852	1.2	771.8	771.8	771.9	0.1

[1]Feet above confluence with Ferson Creek

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

OTTER CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Otter Creek (continued)								
AP	14,038[1]	1,210	4,621	0.3	773.1	773.1	773.2	0.1
AQ	14,808[1]	947	1,447	0.9	773.2	773.2	773.3	0.1
AR	15,358[1]	342	487	2.6	773.9	773.9	773.9	0.0
AS	15,778[1]	251	539	2.3	775.3	775.3	775.3	0.0
AT	16,502[1]	41	287	4.0	777.0	777.0	777.1	0.1
AU	16,682[1]	554	1,403	0.8	777.4	777.4	777.5	0.1
AV	17,229[1]	40	343	3.4	781.0	781.0	781.0	0.0
AW	17,469[1]	465	2,033	0.6	781.3	781.3	781.3	0.0
AX	20,334[1]	1,406	6,059	0.1	781.3	781.3	781.3	0.0
AY	20,914[1]	436	1,439	0.6	781.3	781.3	781.3	0.0
AZ	21,550[1]	374	1,248	0.5	783.8	783.8	783.8	0.0
BA	21,720[1]	354	1,042	0.6	783.8	783.8	783.8	0.0
BB	22,670[1]	368	604	1.1	784.1	784.1	784.1	0.0
BC	22,750[1]	425	527	1.3	784.2	784.2	784.2	0.0
BD	24,150[1]	332	378	1.8	787.5	787.5	787.5	0.0
BE	24,695[1]	162	288	2.3	789.2	789.2	789.2	0.0
BF	24,815[1]	161	251	2.7	789.5	789.5	789.5	0.0
BG	25,415[1]	218	300	1.5	791.0	791.0	791.0	0.0
BH	26,815[1]	59	141	3.1	793.1	793.1	793.1	0.0
BI	26,950[1]	66	138	3.2	793.3	793.3	793.3	0.0
BJ	27,920[1]	41	123	3.6	795.0	795.0	795.0	0.0
Otter Creek West								
A	800[2]	105	150	3.0	760.7	760.7	760.8	0.1
B	1,422[2]	167	325	1.4	765.9	765.9	766.0	0.1

[1]Feet above confluence with Ferson Creek
[2]Feet above confluence with Otter Creek

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

OTTER CREEK - OTTER CREEK WEST

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE [1]	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Otter Creek West (continued)								
C	1,867	15	87	5.2	767.1	767.1	767.1	0.0
D	2,237	38	128	3.5	768.5	768.5	768.6	0.1
E	2,967	91	114	4.0	775.6	775.6	775.6	0.0
F	3,074	162	299	1.5	776.5	776.5	776.6	0.1
G	3,296	121	331	1.4	778.6	778.6	778.6	0.0
H	3,456	178	211	2.1	778.7	778.7	778.7	0.0
I	3,712	16	60	7.5	779.3	779.3	779.4	0.1
J	4,642	136	403	1.1	786.4	786.4	786.4	0.0
K	4,692	105	226	2.0	786.4	786.4	786.4	0.0
L	4,872	116	191	2.4	786.8	786.8	786.8	0.0
M	5,156	119	193	2.3	788.2	788.2	788.2	0.0
N	5,316	120	173	2.6	788.8	788.8	788.8	0.0
O	6,726	116	207	2.2	798.0	798.0	798.0	0.0
P	7,076	96	182	2.5	799.3	799.3	799.3	0.0
Q	8,176	179	431	1.0	805.2	805.2	805.2	0.0
R	8,281	128	264	1.7	805.2	805.2	805.2	0.0
S	8,447	89	182	2.5	805.6	805.6	805.6	0.0
T	8,647	103	183	2.1	805.9	805.9	805.9	0.0
U	9,177	55	96	4.1	807.0	807.0	807.0	0.0
V	9,567	86	81	4.8	809.5	809.5	809.5	0.0
W	10,237	248	327	1.2	812.8	812.8	812.8	0.0
X	11,067	266	245	1.6	815.2	815.2	815.3	0.1
Y	11,657	85	122	3.2	819.4	819.4	819.4	0.0
Z	12,237	765	3,275	0.1	837.5	837.5	837.6	0.1
AA	13,012	290	627	0.5	837.5	837.5	837.6	0.1

[1]Feet above confluence with Otter Creek

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

OTTER CREEK WEST

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Otter Creek West (continued)								
AB	13,477[1]	293	644	0.5	837.6	837.6	837.7	0.1
AC	13,897[1]	98	82	3.6	839.6	839.6	839.6	0.0
Pingree Creek								
A	450[2]	1,375	2,851	0.5	893.4	893.4	893.5	0.1
B	2,915[2]	393	557	2.6	896.0	896.0	896.0	0.0
C	3,744[2]	74	420	3.4	897.7	897.7	897.7	0.0
D	5,221[2]	524	1,375	1.0	898.2	898.2	898.2	0.0
E	6,558[2]	190	589	2.2	899.4	899.4	899.5	0.1
F	7,031[2]	70	461	2.9	899.9	899.9	900.0	0.1
G	7,575[2]	58	401	3.3	900.7	900.7	900.8	0.1
H	9,666[2]	64	344	3.8	900.9	900.9	901.0	0.1
I	10,837[2]	410	944	1.4	901.6	901.6	901.7	0.1
J	11,539[2]	828	4,197	0.3	901.7	901.7	901.8	0.1
K	13,702[2]	390	557	2.3	902.7	902.7	902.8	0.1
L	14,179[2]	42	262	5.0	904.3	904.3	904.4	0.1
Poplar Creek								
A	1,305[3]	159	437	4.6	706.2	706.2	706.2	0.0
B	2,075[3]	463	1,546	1.3	708.1	708.1	708.2	0.1
C	2,350[3]	308	1,182	1.7	708.1	708.1	708.2	0.1
D	3,980[3]	680	1,300	1.5	710.2	710.2	710.3	0.1
E	4,460[3]	763	1,773	1.1	710.2	710.2	710.3	0.1

[1]Feet above confluence with Otter Creek
[2]Feet above confluence with Tyler Creek
[3]Feet above confluence with Fox River

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

OTTER CREEK WEST - PINGREE CREEK - POPLAR CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Prestbury Branch								
A	552[1]	220	672	0.2	678.2	678.2	678.3	0.1
B	686[1]	186	1,093	0.2	678.2	678.2	678.3	0.1
C	2,903[1]	222	101	1.3	686.2	686.2	686.2	0.0
D	3,228[1]	219	570	0.4	687.6	687.6	687.6	0.0
E	8,526[1]	647	3,294	0.0	687.7	687.7	687.7	0.0
Route 38 Branch								
A	1,086[1]	49	62	1.6	833.9	833.9	833.9	0.0
B	1,809[1]	257	164	1.1	835.3	835.3	835.3	0.0
C	2,158[1]	369	2,539	0.1	845.4	845.4	845.4	0.0
Sandy Creek								
A	709[2]	177	390	1.8	796.2	795.9[3]	796.0	0.1
B	1,450[2]	162	388	1.9	798.4	798.4	798.4	0.0
C	5,240[2]	1,020	1,811	0.4	808.6	808.6	808.7	0.1
D	7,250[2]	168	420	1.7	817.9	817.9	818.0	0.1
E	7,646[2]	1,340	3,885	0.1	825.5	825.5	825.5	0.0
F	8,977[2]	103	172	3.2	829.3	829.3	829.3	0.0
G	9,278[2]	132	233	2.4	831.6	831.6	831.6	0.0
H	10,033[2]	137	176	2.6	837.2	837.2	837.2	0.0
I	10,487[2]	120	132	3.5	840.3	840.3	840.3	0.0
J	10,944[2]	185	148	3.1	844.1	844.1	844.1	0.0
K	11,075[2]	134	213	2.2	845.1	845.1	845.1	0.0

[1]Feet above confluence with Blackberry Creek
[2]Feet above mouth
[3]Elevation computed without consideration of backwater effects from Tyler Creek

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

PRESTBURY BRANCH - ROUTE 38 BRANCH - SANDY CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Sandy Creek (continued)								
L	11,517[1]	37	142	3.3	847.4	847.4	847.4	0.0
M	13,176[1]	431	378	1.0	848.3	848.3	848.3	0.0
N	13,783[1]	386	209	1.8	852.6	852.6	852.5	0.1
O	14,598[1]	70	124	3.1	859.7	859.7	859.7	0.0
P	15,107[1]	132	116	1.5	862.8	862.8	862.8	0.0
Q	15,616[1]	122	136	1.3	865.7	865.7	865.7	0.0
R	16,185[1]	21	47	3.8	872.6	872.6	872.6	0.0
S	16,936[1]	41	52	3.4	882.6	882.6	882.6	0.0
T	17,490[1]	82	102	1.8	886.5	886.5	886.5	0.0
U	17,918[1]	109	112	0.6	888.4	888.4	888.4	0.0
Seavey Road Run								
A	1,812[2]	307	818	2.1	708.3	708.3	708.4	0.1
B	2,198[2]	340	1,613	1.0	709.1	709.1	709.1	0.1
C	4,470[2]	198	441	3.1	710.8	710.8	710.8	0.0
D	4,972[2]	484	1,987	0.7	712.2	712.2	712.2	0.0
E	5,580[2]	390	1,239	1.4	712.3	712.3	712.4	0.0
F	7,070[2]	129	452	2.7	713.5	713.5	713.4	0.0
G	7,384[2]	250	746	2.4	715.1	715.1	715.0	0.0
H	11,386[2]	365	868	2.2	717.8	717.8	717.7	0.0
I	13,407[2]	174	486	2.6	720.0	720.0	720.0	0.0
J	13,683[2]	240	893	1.6	721.2	721.2	721.2	0.1
K	19,650[2]	155	203	3.0	724.1	724.1	724.1	0.0

[1]Feet above mouth
[2]Feet above confluence with Blackberry Creek

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

SANDY CREEK - SEAVEY ROAD RUN

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Seavey Road Run (continued)								
L	20,615[1]	48	34	3.2	731.7	731.7	731.7	0.0
M	22,201[1]	20	26	3.1	750.5	750.5	750.5	0.0
N	23,095[1]	102	56	1.9	762.5	762.5	762.5	0.0
O	23,474[1]	180	318	0.3	766.4	766.4	766.4	0.0
P	24,228[1]	143	68	0.8	768.6	768.6	768.6	0.0
Seavey Road Run Green Road Branch								
A	1,365[2]	367	488	0.7	727.5	727.5	727.5	0.0
B	2,875[2]	161	167	0.8	731.9	731.9	731.9	0.0
Seavey Road Run Main Street Branch								
A	2,785[2]	453	584	1.7	727.4	727.4	727.4	0.0
B	4,817[2]	106	108	2.9	739.5	739.5	739.5	0.0
C	5,816[2]	86	119	3.1	746.9	746.9	746.9	0.0
D	6,124[2]	345	545	1.0	750.4	750.4	750.4	0.0

[1]Feet above confluence with Blackberry Creek
[2]Feet above confluence with Seavey Road Run

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

SEAVEY ROAD RUN - SEAVEY ROAD RUN GREEN ROAD BRANCH - SEAVEY ROAD RUN MAIN STREET BRANCH

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Selmarten Creek								
A	143[1]	695	380	1.0	714.7	714.7	714.8	0.1
B	364[1]	546	360	1.1	714.7	714.7	714.8	0.1
C	876[1]	905	401	1.0	714.7	714.7	714.8	0.1
D	1,135[1]	1,162	261	1.2	714.7	714.7	714.8	0.1
E	3,384[1]	208	143	2.0	715.5	715.5	715.4	0.0
F	3,791[1]	107	39	6.6	716.6	716.6	716.5	0.0
G	4,726[1]	542	326	0.7	717.4	717.4	717.4	0.0
Sleepy Creek								
A	1,491[2]	36	120	4.3	719.3	719.3	719.4	0.1
B	3,028[2]	36	161	3.2	727.5	727.5	727.5	0.0
C	3,718[2]	56	193	2.7	727.8	727.8	727.9	0.1
D	5,472[2]	380	367	1.4	731.2	731.2	731.3	0.1
E	5,874[2]	224	454	1.1	734.0	734.0	734.0	0.0
F	7,247[2]	218	185	2.7	736.9	736.9	736.9	0.0
G	8,794[2]	138	357	1.4	742.2	742.2	742.2	0.0
H	9,787[2]	231	308	1.6	743.2	743.2	743.2	0.0
I	10,611[2]	88	87	5.6	749.2	749.2	749.2	0.0
J	12,585[2]	36	74	4.7	767.8	767.8	767.8	0.0

[1]Feet above confluence with Indian Creek
[2]Feet above confluence with Fox River

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

SELMARTEN CREEK - SLEEPY CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
South Tributary								
A	180[1]	697	2,425	3.6	689.0	689.0	689.0	0.0
B	480[1]	267	486	2.6	689.2	689.2	689.2	0.0
C	9,751[1]	73	257	3.4	690.9	690.9	690.9	0.0
D	1,350[1]	51	192	3.4	691.5	691.5	691.6	0.1
E	1,450[1]	87	270	2.4	692.1	692.1	692.2	0.1
F	1,688[1]	102	337	2.0	692.6	692.6	692.7	0.1
G	1,836[1]	137	419	1.6	693.0	693.0	693.1	0.1
H	2,045[1]	175	382	1.7	693.2	693.2	693.3	0.1
I	2,220[1]	370	466	2.1	693.7	693.7	693.8	0.1
J	2,675[1]	275	435	1.0	694.2	694.2	694.3	0.1
K	2,985[1]	60	284	1.5	694.8	694.8	694.8	0.0
L	5,030[1]	169	514	0.8	698.4	698.4	698.5	0.1
M	5,575[1]	28	91	4.2	698.7	698.7	698.8	0.1
State Street Creek								
A	985[2]	*	*	*	695.0	695.0	*	*
B	3,450[2]	*	*	*	721.8	721.8	*	*
State Street Creek Tributary								
A	1,500[3]	*	*	*	740.6	740.6	*	*

[1]Feet above confluence with Indian Creek
[2]Feet above confluence with Fox River
[3]Feet above confluence with State Street Creek
*Data not available

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

SOUTH TRIBUTARY - STATE STREET CREEK - STATE STREET CREEK TRIBUTARY

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE [1]	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Stony Creek								
A	500	203	466	2.1	773.4	773.4	773.5	0.1
B	880	35	160	6.2	773.9	773.9	773.9	0.0
C	1,230	185	335	2.9	775.8	775.8	775.9	0.1
D	2,030	221	480	2.1	778.6	778.6	778.6	0.0
E	3,117	38	177	5.6	781.7	781.7	781.8	0.1
F	3,417	36	136	7.3	783.0	783.0	783.0	0.0
G	4,217	117	300	3.3	787.5	787.5	787.6	0.1
H	5,017	124	301	3.3	790.0	790.0	790.0	0.0
I	6,417	957	1,130	0.9	792.2	792.2	792.2	0.0
J	7,087	564	700	0.6	792.9	792.9	792.9	0.0
K	8,095	94	118	3.7	793.9	793.9	793.9	0.0
L	8,410	44	133	3.3	795.4	795.4	795.5	0.1
M	10,040	199	217	2.0	800.7	800.7	800.7	0.0
N	10,940	57	202	2.1	802.6	802.6	802.7	0.1
O	12,040	34	133	3.1	804.6	804.6	804.7	0.1
P	12,610	38	98	4.1	807.6	807.6	807.6	0.0
Q	13,640	34	109	3.7	816.5	816.5	816.5	0.0
R	15,240	90	217	1.8	823.1	823.1	823.1	0.0
S	16,840	177	293	1.4	826.9	826.9	827.0	0.1
T	17,240	35	98	4.0	828.1	828.1	828.2	0.1
U	18,171	10	66	4.7	832.6	832.6	832.6	0.0
V	18,536	251	613	0.5	833.2	833.2	833.2	0.0
W	18,956	108	123	2.5	833.3	833.3	833.3	0.0
X	19,806	353	389	0.8	836.4	836.4	836.4	0.0
Y	21,056	133	150	2.1	839.4	839.4	839.4	0.0
Z	22,291	225	730	0.4	850.8	850.8	850.8	0.0

[1]Feet above confluence with Otter Creek

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

STONY CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Stony Creek (Continued)								
AA	22,631[1]	229	978	0.3	850.9	850.9	850.9	0.0
AB	23,631[1]	35	47	6.7	850.9	850.9	850.9	0.0
AC	25,031[1]	34	99	3.2	859.6	859.6	859.6	0.0
AD	26,231[1]	32	78	4.0	863.4	863.4	863.4	0.0
AE	27,531[1]	17	49	6.4	872.3	872.3	872.3	0.0
Sugar Grove Branch								
A	647[2]	*	*	*	677.0	677.0	*	*
B	2,639[2]	*	*	*	678.9	678.9	*	*
C	3,795[2]	320	919	1.9	679.8	679.8	679.8	0.0
D	5,705[2]	39	193	7.2	682.4	682.4	682.5	0.1
E	7,330[2]	*	*	*	685.4	685.4	*	*
F	8,760[2]	*	*	*	686.5	686.5	*	*
G	10,910[2]	*	*	*	692.1	692.1	*	*
H	12,950[2]	*	*	*	696.1	696.1	*	*
I	17,115[2]	*	*	*	710.9	710.9	*	*
Sugar Grove Branch East								
A	3,165[3]	*	*	*	693.4	693.4	*	*
B	5,300[3]	*	*	*	694.8	694.8	*	*

[1]Feet above confluence with Otter Creek
[2]Feet above mouth at Welch Creek
[3]Feet above confluence with Sugar Grove Branch
*Data not available

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

STONY CREEK - SUGAR GROVE BRANCH - SUGAR GROVE BRANCH EAST

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Sugar Grove Branch North								
A	1,160[1]	*	*	*	686.6	686.6	*	*
B	2,170[1]	*	*	*	689.7	689.7	*	*
C	2,915[1]	*	*	*	692.3	692.3	*	*
Tollway Tributary	*	*	*	*	*	*	*	*
Tyler Creek								
A	375[2]	131	495	6.9	715.9	715.9	715.9	0.0
B	1,045[2]	255	1,191	2.8	717.9	717.9	717.9	0.0
C	1,555[2]	500	1,502	4.4	718.6	718.6	718.7	0.1
D	2,015[2]	700	1,835	2.8	720.8	720.8	720.8	0.0
E	2,875[2]	115	451	7.6	726.5	726.5	726.6	0.1
F	3,645[2]	256	1,764	1.9	732.8	732.8	732.8	0.0
G	4,605[2]	306	784	4.3	735.2	735.2	735.3	0.1
H	5,195[2]	252	939	3.6	740.5	740.5	740.5	0.0
I	7,935[2]	130	756	4.5	756.3	756.3	756.3	0.0
J	9,205[2]	390	3,173	1.0	774.6	774.6	774.6	0.0
K	10,275[2]	325	3,102	1.0	776.7	776.7	776.7	0.0
L	10,575[2]	610	3,844	0.9	776.7	776.7	776.7	0.0
M	11,035[2]	503	2,777	1.2	776.9	776.9	776.9	0.0
N	12,065[2]	114	802	4.1	778.1	778.1	778.1	0.0
O	14,135[2]	151	550	6.2	785.2	785.2	785.3	0.1
P	16,215[2]	230	1,615	2.1	793.1	793.1	793.1	0.0

[1]Feet above confluence with Sugar Grove Branch
[2]Feet above confluence with Fox River
*Data not available

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

SUGAR GROVE BRANCH NORTH - TOLLOWAY TRIBUTARY - TYLER CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Tyler Creek (continued)								
Q	18,175[1]	345	1,524	2.3	794.4	794.4	794.4	0.0
R	20,725[1]	255	1,107	2.5	797.5	797.5	797.6	0.1
S	21,483[1]	520	2,341	1.2	801.0	801.0	801.0	0.0
T	24,355[1]	455	1,166	2.4	803.1	803.1	803.1	0.0
U	26,307[1]	190	1,144	2.4	807.5	807.5	807.6	0.1
V	27,345[1]	450	757	3.6	808.9	808.9	808.9	0.0
W	27,785[1]	600	2,367	1.1	811.1	811.1	811.1	0.0
X	30,864[1]	303	969	2.7	814.0	814.0	814.0	0.0
Y	31,060[1]	349	1,116	2.4	814.5	814.5	814.6	0.1
Z	33,941[1]	461	1,256	2.1	820.5	820.5	820.6	0.1
AA	35,224[1]	296	863	3.1	824.6	824.6	824.7	0.1
AB	36,581[1]	241	847	3.1	830.6	830.6	830.6	0.0
AC	37,988[1]	450	1,361	1.9	835.2	835.2	835.3	0.1
AD	38,528[1]	440	3,368	0.8	845.1	845.1	845.1	0.0
AE	40,264[1]	76	473	5.6	846.3	846.3	846.3	0.0
AF	40,420[1]	100	1,331	2.0	853.8	853.8	853.8	0.0
AG	43,087[1]	242	833	3.2	859.4	859.4	859.5	0.1
AH	44,215[1]	70	331	7.4	864.7	864.7	864.7	0.0
AI	44,351[1]	310	1,629	1.5	867.0	867.0	867.1	0.1
AJ	46,202[1]	165	641	3.8	870.5	870.5	870.5	0.0
AK	48,137[1]	150	466	5.3	874.6	874.6	874.7	0.1
AL	50,160[1]	435	1,148	2.1	879.1	879.1	879.2	0.1
AM	50,458[1]	50	353	6.8	881.8	881.8	881.8	0.0

[1]Feet above confluence with Fox River

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Tyler Creek (continued)								
AN	55,530[1]	263	826	2.7	885.7	885.7	885.8	0.1
AO	57,475[1]	100[2]	479	4.6	887.8	887.8	887.8	0.0
AP	57,951[1]	615	1,709	1.3	888.8	888.8	888.8	0.0
AQ	62,816[1]	1,900	2,154	1.0	890.0	890.0	890.1	0.1
AR	64,082[1]	1,395	1,826	1.1	891.8	891.8	891.8	0.0
AS	64,500[1]	600	546	3.8	892.3	892.3	892.3	0.0
AT	65,541[1]	38	212	3.2	894.2	894.2	894.2	0.0
AU	66,630[1]	49	289	2.3	896.0	896.0	896.0	0.0
AV	67,158[1]	63	348	1.9	896.4	896.4	896.4	0.0
AW	71,140[1]	40	272	2.0	896.7	896.7	896.7	0.0
Tyler Creek Unnamed Tributary	*	*	*	*	*	*	*	*
Union Ditch No. 2	*	*	*	*	*	*	*	*
Waubonsee Creek								
A	30,435[1]	1,062	2,983	0.9	666.3	666.3	666.4	0.1
B	31,865[1]	601	1,239	1.4	666.7	666.7	666.7	0.0
C	35,635[1]	65	*	*	669.4	669.4	669.4	0.0
D	36,525[1]	95	*	*	670.0	670.0	670.0	0.0
E	37,215[1]	60	*	*	670.3	670.3	670.3	0.0

[1]Feet above confluence with Fox River

[2]Floodway width reflects constricted section, see FIRM panel for regulatory floodway

*Data not available

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

TYLER CREEK - TYLER CREEK UNNAMED TRIBUTARY - UNION DITCH NO. 2 - WAUBONSEE CREEK

FLOODING SOURCE		FLOODWAY			1-PERCENT-ANNUAL-CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE [1]	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE (FEET)
Welch Creek								
A	2,440	*	*	*	666.8	666.8	*	*
B	8,678	*	*	*	671.5	671.5	*	*
C	16,644	*	*	*	685.5	685.5	*	*
D	18,150	*	*	*	689.2	689.2	*	*
E	21,281	*	*	*	691.3	691.3	*	*
F	28,526	*	*	*	701.1	701.1	*	*
G	32,060	*	*	*	706.7	706.7	*	*
H	40,564	*	*	*	725.9	725.9	*	*
I	47,792	*	*	*	742.3	742.3	*	*
J	63,515	*	*	*	774.9	774.9	*	*
K	67,002	*	*	*	784.0	784.0	*	*
L	68,253	*	*	*	786.7	786.7	*	*
M	74,380	*	*	*	792.4	792.4	*	*
N	79,054	*	*	*	793.5	793.5	*	*
O	81,584	*	*	*	795.7	795.7	*	*
P	89,053	*	*	*	813.2	813.2	*	*
West Branch Big Rock Creek								
A	3,077	*	*	*	692.2	692.2	*	*
B	6,929	*	*	*	702.5	702.5	*	*
C	13,150	*	*	*	716.3	716.3	*	*

[1]Feet above confluence with Big Rock Creek
*Data not available

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

FLOODWAY DATA

WELCH CREEK – WEST BRANCH BIG ROCK CREEK

5.0 INSURANCE APPLICATIONS

For flood insurance rating purposes, flood insurance zone designations are assigned to a community based on the results of the engineering analyses. The zones are as follows:

Zone A

Zone A is the flood insurance rate zone that corresponds to the 1-percent-annual-chance floodplains that are determined in the FIS by approximate methods. Because detailed hydraulic analyses are not performed for such areas, no base flood elevations or depths are shown within this zone.

Zone AE

Zone AE is the flood insurance rate zone that corresponds to the 1-percent-annual-chance floodplains that are determined in the FIS by detailed methods. In most instances, whole-foot base flood elevations derived from the detailed hydraulic analyses are shown at selected intervals within this zone.

Zone AH

Zone AH is the flood insurance rate zone that corresponds to the areas of 1-percent-annual-chance shallow flooding (usually areas of ponding) where average depths are between 1 and 3 feet. Whole-foot base flood elevations derived from the detailed hydraulic analyses are shown at selected intervals within this zone.

Zone AO

Zone AO is the flood insurance rate zone that corresponds to the areas of 1-percent-annual-chance shallow flooding (usually sheet flow on sloping terrain) where average depths are between 1 and 3 feet. Average whole-foot depths derived from the detailed hydraulic analyses are shown within this zone.

Zone X

Zone X is the flood insurance rate zone that corresponds to areas outside the 0.2-percent-annual-chance floodplain, areas within the 0.2-percent-annual-chance floodplain, and to areas of 1-percent-annual-chance flooding where average depths are less than 1 foot, areas of 1-percent-annual-chance flooding where the contributing drainage area is less than 1 square mile, and areas protected from the 1-percent-annual-chance flood by levees. No base flood elevations or depths are shown within this zone.

6.0 FLOOD INSURANCE RATE MAP

The FIRM is designed for flood insurance and floodplain management applications.

For flood insurance applications, the map designates flood insurance rate zones as described in Section 5.0 and, in the 1-percent-annual-chance floodplains that were studied by detailed methods, shows selected whole foot base flood elevations or average depths. Insurance agents use the zones and base flood elevations in conjunction with information on structures and their contents to assign premium rates for flood insurance policies.

For floodplain management applications, the map shows by tints, screens, and symbols, the 1- and 0.2-percent-annual-chance floodplains. Floodways and the locations of selected cross sections used in the hydraulic analyses and floodway computations are shown where applicable.

The current FIRM presents flooding information for the entire geographic area of Kane County. Previously, separate Flood Hazard Boundary Maps and/or FIRMs were prepared for each identified flood prone incorporated community and the unincorporated areas of the Kane County. The countywide FIRM also includes flood hazard information that was presented separately on Flood Boundary and Floodway Maps (FBFM), where applicable. Historical data relating to the community maps prepared is presented in Table 14, "Community Map History."

7.0 OTHER STUDIES

FISs have been prepared for McHenry, Kendall, DeKalb, Cook, and DuPage Counties, Illinois (Reference 93).

Information pertaining to revised and unrevised flood hazards for each jurisdiction, or portion of each jurisdiction, located within Kane County has been compiled into this FIS. Therefore, this FIS supersedes all previously printed FIS Reports, FHBMs, FIRMs, and/or FBFMs for all the incorporated and unincorporated jurisdictions within Kane County.

8.0 LOCATION OF DATA

Information concerning the pertinent data used in the preparation of this FIS can be obtained by contacting FEMA, Federal Insurance and Mitigation Division, 536 South Clark Street, Sixth Floor, Chicago, Illinois 60605.

COMMUNITY NAME	INITIAL IDENTIFICATION	FLOOD HAZARD BOUNDARY MAP REVISION DATE(S)	FLOOD INSURANCE RATE MAP EFFECTIVE DATE	FLOOD INSURANCE RATE MAP REVISION DATE(S)
Algonquin, Village of	March 8, 1974	March 26, 1976	March 16, 1981	None
Aurora, City of	June 14, 1974	September 24, 1976	June 15, 1979	May 15, 1986 January 5, 1989 March 3, 1997
Barrington Hills, Village of [1]	April 5, 1974	July 9, 1976	August 10, 1979	None
Bartlett, Village of	April 12, 1974	August 6, 1976	June 15, 1981	March 15, 1984
Batavia, City of	September 10, 1976	None	September 2, 1981	None
Big Rock, Village of	May 14, 1976 [2]	None	March 1, 1982 [2]	June 4, 1996 [2] May 19, 1997 [2]
Burlington, Village of [3]	N/A	None	N/A	None
Campton Hills, Village of	May 14, 1976 [2]	None	March 1, 1982 [2]	June 4, 1996 [2] May 19, 1997 [2]
Carpentersville, Village of	March 22, 1974	January 9, 1976	August 17, 1981	None
East Dundee, Village of	May 17, 1974	January 2, 1976	March 16, 1981	June 1, 1984

[1] No Special Flood Hazard Areas identified in Kane County. Map dates are from Cook County.
[2] Date from Kane County, Unincorporated Areas, Flood Insurance Rate Map
[3] No Special Flood Hazard Areas identified
N/A = Not applicable

TABLE 14

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

COMMUNITY MAP HISTORY

COMMUNITY NAME	INITIAL IDENTIFICATION	FLOOD HAZARD BOUNDARY MAP REVISION DATE(S)	FLOOD INSURANCE RATE MAP EFFECTIVE DATE	FLOOD INSURANCE RATE MAP REVISION DATE(S)
Elburn, Village of	December 20, 2002	None	December 20, 2002	None
Elgin, City of	May 3, 1974	October 31, 1975 June 23, 1978	March 1, 1982	April 17, 1984 February 17, 1989
Geneva, City of	August 9, 1974	January 23, 1976	August 3, 1981	None
Gilberts, Village of	December 20, 2002	None	December 20, 2002	None
Hampshire, Village of	May 3, 1974	March 26, 1976	March 2, 1981	November 2, 1995
Hoffman Estates, Village of [2]	September 20, 1974	August 27, 1976	May 19, 1981	November 20, 1991
Huntley, Village of	March 29, 1974	February 6, 1976	December 15, 1992	May 19, 1997
Kane County (Unincorporated Areas)	May 14, 1976	None	March 1, 1982	June 4, 1996 May 19, 1997
Kaneville, Village of [1]	N/A	None	N/A	None
Lily Lake, Village of	May 14, 1976 [3]	None	June 16, 1992	None
Maple Park, Village of	August 4, 1987	None	August 4, 1987	None

[1] No Special Flood Hazard Areas identified
[2] No Special Flood Hazard Areas identified in Kane County. Map dates are from Cook County.
[3] Date from Kane County, Unincorporated Areas, Flood Insurance Rate Map
N/A = Not applicable

TABLE 14

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

COMMUNITY MAP HISTORY

COMMUNITY NAME	INITIAL IDENTIFICATION	FLOOD HAZARD BOUNDARY MAP REVISION DATE(S)	FLOOD INSURANCE RATE MAP EFFECTIVE DATE	FLOOD INSURANCE RATE MAP REVISION DATE(S)
Montgomery, Village of	October 26, 1973	May 28, 1976	August 15, 1979	None
North Aurora, Village of	March 1, 1974	July 9, 1976 February 15, 1980	March 16, 1981	None
Pingree Grove, Village of	December 20, 2002	None	December 20, 2002	None
Sleepy Hollow, Village of	April 12, 1974	April 9, 1976	June 15, 1982	None
South Elgin, Village of	April 5, 1974	April 23, 1976 July 7, 1978	July 16, 1981	None
St. Charles, City of	March 15, 1974	June 4, 1976 December 17, 1976	September 2, 1981	None
Sugar Grove, Village of	September 30, 1976	None	September 30, 1976	March 4, 1988
Virgil, Village of	May 14, 1976 [1]	None	June 2, 1992	None
Wayne, Village of	August 15, 1975	None	December 1, 1981	None
West Dundee, Village of	April 5, 1974	March 19, 1976	December 1, 1981	June 1, 1984

[1] Date from Kane County, Unincorporated Areas, Flood Insurance Rate Map

TABLE 14

FEDERAL EMERGENCY MANAGEMENT AGENCY

KANE COUNTY, IL AND INCORPORATED AREAS

COMMUNITY MAP HISTORY

9.0 BIBLIOGRAPHY AND REFERENCES

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32. Federal Emergency Management Agency, Federal Insurance Administration. *Flood Insurance Study: Village of West Dundee, Illinois, Kane County.* Washington, D.C.: June 1, 1981.

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